

American Electric Power Service Corporation

2018 Landfill Lateral Expansion - CCR Location Restriction Evaluation

H. W. Pirkey Power Plant 2400 FM 3251 Harrison County Hallsville, Texas

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H.W. Pirkey Power Plant 2400 FM 3251 Harrison County Hallsville, Texas

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Acronyms and Abbreviation

AEP	American Electric Power Service Cooperation
amsl	above mean sea level

ARCADIS U.S., Inc. BAP bottom ash pond

BGL below ground surface

CCR Coal Combustion Residual

CFR Code of Federal Regulations



EPRI Electric Power Research Institute

FAP fly ash pond

FGD flue gas desulfurization

ft feet

PTI Permit to Install

TAC Texas Administrative Code

TCEQ Texas Commission on Environmental Quality



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1. Objective

This report was prepared by ARCADIS U.S., Inc. (ARCADIS) for American Electric Power Service Corporation (AEP) to assess the location of the 2018 Lateral Expansion of the existing on-site Landfill relative to the location restrictions included in the Coal Combustion Residual (CCR) requirements, as specified in the Code of Federal Regulations (CFR) 40 CFR 257.60 to 257.64, at the AEP H.W. Pirkey Generating Plant (Plant) located at 2400 FM 3251 in Hallsville, Harrison County, Texas (**Figure 1**). The CCR requirements include an evaluation of the adequacy of the groundwater monitoring well network to characterize groundwater quality up and down gradient of the CCR unit and an evaluation of whether the CCR unit meets up to 5 location restrictions, which include: the base of the CCR unit is 5 feet (ft) above the uppermost aquifer, the CCR unit may not be located in a wetland, within 200 ft of the damage zone of a fault that has displacement during the Holocene, within a seismic impact zone, or in an unstable area.

Four regulated CCR units associated with the Plant were identified for review, which include the West Bottom Ash Pond (BAP), East BAP, Stack Out Area, and Landfill (**Figure 2**). The initial CCR Location Restriction Evaluation for the existing Landfill was prepared in 2016 (Arcadis, 2016). This report has been prepared for the 2018 Lateral Expansion of the existing Landfill (Site) and includes an updated evaluation of the location restriction criteria. The evaluation of the groundwater monitoring well network in the uppermost aquifer for the 2018 Lateral Expansion of the existing Landfill is not included in this report and will be completed under separate cover.

This evaluation included a review of AEP-provided data associated with previously completed subsurface investigation activities in the vicinity of the Landfill CCR unit, as well as publicly-available geologic and hydrogeologic data. The following report also presents the current Conceptual Site Model based on documents reviewed and will further describe the uppermost aquifer.



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2. Background Information

The following section provides background information for the AEP H.W. Pirkey Generating Plant Landfill.

2.1 Facility Location Description

The AEP H.W. Pirkey Plant is located in southern Harrison County, approximately 5 miles southeast of Hallsville, Texas, and approximately 8 miles southwest of Marshall, Texas (**Figure 1**). The existing Landfill CCR unit is currently approximately 130 acres in size and is located in the southern portion of the Plant. The 2018 Landfill Lateral Expansion will cover approximately 15 acres directly south of the existing Landfill (**Figures 2** and **3**). Following completion of the 2018 Lateral Expansion, the Landfill will be approximately 145 acres in size.

2.2 Description of Landfill CCR Unit

The following section will discuss the embankment configuration, area, volume, construction and operational history, and surface water control associated with the Landfill.

2.2.1 Embankment Configuration

The Landfill was constructed in the southwestern portion of the Plant, and as shown on the U.S. Geological Survey 1983 topographic map (**Figure 1**), the southwestern portion of the Plant contained an unnamed intermittent tributary of Hatley Creek prior to Landfill construction in 1984. The Landfill was constructed within the unnamed tributary creek which had a bottom elevation ranging from approximately 290 feet amsl on the south side of the Landfill to 300 feet amsl on the north side of the Landfill. The native soil sidewalls of the tributary creek at the Landfill location have a maximum elevation of approximately 355 feet amsl. Therefore, as shown on Geologic Cross Section C-C' (**Figure 6**), the Landfill is partially incised within the tributary creek, and the tributary creek native soil sidewalls serve as a natural embankment for the lower portion of the Landfill.

The original Landfill design included emplacement of CCR materials in the Landfill with 3:1 slopes (3 feet horizontal, 1 foot vertical) with an approximate 10 foot wide bench for every 20 foot vertical rise of CCR material (VFL Technology Corporation, 1984). Apex Geoscience conducted a geotechnical investigation of the Landfill CCR materials in 2012, and concluded the CCR material embankments would be stable on 3:1 slopes (Apex Geoscience, 2013).

In addition to the Landfill CCR material embankments, earthen embankments are present around portions of the Landfill to control storm water flow. The earthen embankments are constructed using compacted clay on 3:1 slopes. ETTL Engineers & Consultants Inc (ETTL) conducted a geotechnical evaluation of the perimeter



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embankments in 2005 and concluded the perimeter embankments would be stable on slopes no steeper than 3:1 unless the slopes are stabilized with geogrid reinforcement (ETTL, 2005).

The Landfill Stormwater Runoff Pond (non-CCR Unit) was constructed downslope (south) of the Landfill in 1993 and 1994. The Landfill Stormwater Runoff Pond has perimeter compacted soil embankments up to approximately 19 feet in height. Apex Geoscience conducted a geotechnical investigation of the Landfill Stormwater Runoff Pond embankments in 2011. Apex Geoscience recommended raising the elevation of the Landfill Stormwater Runoff Pond embankments to an elevation of 302 feet amsl using soils compacted to at least 95% standard proctor density, with an emergency spillway at a crest elevation of 298 feet amsl. The Apex Geoscience report concluded the Landfill Stormwater Runoff Pond embankments would be stable (Apex Geoscience, April 2011).

The 2018 Landfill Lateral Expansion will cover approximately 15 acres directly south of the existing Landfill. In 2016, Auckland Consulting conducted a stability assessment of the 2018 Lateral Expansion area and concluded the embankments would be stable on slopes no steeper than 3:1 (Auckland, November 2016). The 2016 Auckland Consulting report stated the northern and eastern extents of the 2018 Lateral Expansion will be constructed contiguous to the existing Landfill disposal area, and ash disposal will be completed in multiple lifts, each with an approximate height of 20 feet, integrated with safety benches, and maximum side slopes of 3:1 to a maximum waste height of 120 feet.

2.2.2 Area/Volume

The existing Landfill is approximately 130 acres in size and was designed to receive 12,207,000 cubic yards (7,566 acre feet) of CCR materials including fly ash, bottom ash, economizer ash, and stabilized FGD sludge (VFL Technology Corporation, 1984). The design maximum CCR material height in the existing Landfill is approximately 140 feet (Apex Geoscience, 2013). As discussed above in Section 2.2.1, the 2018 Landfill Lateral Expansion will cover an area of approximately 15 acres to a maximum waste height of 120 feet. The design capacity of the CCR materials to be placed within the 2018 Lateral Expansion is approximately 2,200,000 cubic yards.

2.2.3 Construction and Operational History

The H.W. Pirkey Power Plant was constructed in 1983 and 1984, and began operation in 1985. Throughout the life of the Plant, CCR materials (fly ash, bottom ash, economizer ash, stabilized FGD sludge) have been generated. The CCR materials that are not taken offsite for beneficial reuse are disposed of in the Landfill. The Landfill was constructed in several phases beginning with the northeast portion (Phase 1) in 1984. The Landfill was expanded (east-central portion) in 1985 and 1987. The Landfill was subsequently expanded to the west and south during the 1990's, including



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construction of the Landfill Stormwater Runoff Pond (non-CCR unit) directly south of the Landfill in 1993 and 1994. The Landfill was further expanded to the west between 2005 and 2015 to its current size of approximately 130 acres as shown on **Figure 3**.

In 2005, ETTL conducted a geotechnical evaluation of the Landfill and Landfill Stormwater Runoff Pond, including installation of 30 soil borings, ten cone penetration test (CPT) borings, and geotechnical testing of soil samples. The ETTL report concluded the Landfill CCR materials would be stable at 3:1 slopes; and foundation settlement would be within acceptable limits (ETTL, 2005). The ETTL report recommended that Landfill expansion activities include a composite liner system consisting of a 2-foot-thick compacted clay liner or geosynthetic clay liner (GCL) as the bottom component; and a top liner component consisting of a PVC, high-density polyethylene (HDPE), or a very low density polyethylene (VLDPE) liner. ETTL also recommended Landfill expansion activities include installation of a Landfill leachate collection system consisting of permeable bottom ash emplaced above the Landfill liner. These recommendations were implemented during Landfill expansion phases between 2005 and 2015.

The 2018 Landfill Lateral Expansion design includes emplacement of up to 120 feet of CCR materials with maximum side slopes of 3:1 above the Landfill liner system which will consist of a 60-mil HDPE top liner underlain by a 2-foot-thick compacted clay bottom liner. Prior to installation of the liner system, approximately 10 to 15 feet of cut into the existing soils will occur along the topographically higher southern portion of the Lateral Expansion, and emplacement of these soils (clayey sands, silty clayey sand and/or lean clays) as fill materials below the liner system in the topographically lower central and northern portions of the Lateral Expansion. As detailed below in Section 3, the fill materials emplaced in the topographically lower portions of the Lateral Expansion will extend in height at least five feet above the water table, thereby providing the required separation distance of at least five feet between the base of the CCR unit and the top of the uppermost aquifer.

2.2.4 Surface Water Control

Surface water in the area of the existing Landfill flows in a general southerly direction to the Landfill Stormwater Runoff Pond located directly southwest of the existing Landfill. Surface water in the area of the 2018 Landfill Lateral Expansion flows in a general westerly direction to the Landfill Stormwater Runoff Pond. The Landfill Stormwater Runoff Pond, which is approximately 16 acres in size, also receives (1) Landfill leachate that is gravity drained from the existing Landfill via underground lateral perforated pipes and permeable bottom ash materials that were installed above portions of the existing Landfill liner, (2) Landfill leachate that will gravity drain via underground lateral perforated pipes that will be installed above the 2018 Landfill Lateral Expansion liner system, and (3) shallow groundwater that will gravity drain via underground lateral perforated HDPE underdrain pipes that will be installed in



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permeable bottom ash materials approximately seven feet below the 60-mil HDPE liner of the 2018 Landfill Lateral Expansion.

An emergency spillway is present at the southern end of the Landfill Stormwater Runoff Pond at an elevation of approximately 298 feet amsl. The top of the Landfill Stormwater Runoff Pond is located at an elevation of approximately 302 feet amsl, therefore the Landfill Stormwater Runoff Pond has approximately four feet of freeboard (Apex Geoscience, April 2011). Water in the Landfill Stormwater Runoff Pond discharges into an unnamed intermittent tributary of Hatley Creek via Outfall 004 in accordance with Texas Pollutant Discharge Elimination System (TPDES) Permit No. WQ0002496000.

2.3 Previous Investigations

The initial soils investigation and design of the Plant was provided in a January 31, 1983 report prepared by Sargent & Lundy entitled "Henry W. Pirkey Power Plant, Design Summary for Lignite Storage Area and Wastewater Pond Facilities". This investigation included advancement of soil borings throughout the Plant, including the Landfill Area.

A soils investigation of the Landfill was conducted by Southwestern Laboratories in 1984. The investigation included installation of 45 soil borings and geotechnical analyses of soil samples. The report recommended installation of three feet of compacted clay as the bottom liner for the Landfill (Southwestern Laboratories, July 1984).

An engineering design report for the Landfill was prepared by VFL Technology Corporation in 1984. The Landfill design included a bottom compacted clay liner three feet in thickness, and Landfill side slopes of 3:1 (VFL Technology Corporation, 1984).

In 1985, Southwestern Laboratories conducted a geotechnical evaluation of the clay liner that was installed at the base of the Landfill, including installation of four soil borings and permeability testing of soil samples. The report concluded the clay liner was three feet thick with a permeability less than 1×10^{-7} centimeters per second (cm/sec) (Southwestern Laboratories, 1985).

In 1993, Alliance Inc. conducted a geotechnical investigation of the clay liner installed at the base of the Landfill following a Landfill expansion phase in 1993. The report concluded the clay liner was three feet or more in thickness, and the clay liner met the permeability specifications of $<1 \times 10^{-7}$ cm/sec (Alliance Inc., 1993).

In 1995, Central and South West Services prepared design specifications for Landfill expansion to the west and south. The design specifications included a geosynthetic clay liner overlain by a 0.060 inch (60 mil) HDPE liner (Central and South West Services, 1995).



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In 2005, ETTL conducted a geotechnical evaluation of the Landfill and Landfill Stormwater Runoff Pond, including installation of 30 soil borings, ten CPT borings, and geotechnical testing of soil samples. The geotechnical data was obtained to design Landfill expansions in 2005 through 2007. The ETTL report concluded the Landfill CCR materials would be stable at 3:1 slopes; and foundation settlement would be within acceptable limits (ETTL, 2005). The ETTL report recommended the Landfill expansion include a composite liner system consisting of a 2-foot-thick compacted clay liner or GCL as the bottom component; and a top liner component consisting of a PVC, HDPE, or a VLDPE liner. ETTL also recommended the Landfill expansion include a leachate collection system consisting of permeable bottom ash emplaced above the Landfill liner. These recommendations were implemented during Landfill expansion phases between 2005 and 2015.

In 2010 and January 2011, Apex Geoscience expanded the groundwater monitoring well system at the Plant, including installation of monitoring wells AD-16 through AD-29. Apex Geoscience also conducted video surveillance of the existing monitoring wells and plugged monitoring wells MW-1, MW-5, MW-6, MW-9, MW-11, MW-14, MW-15, M-2, and M-3 (Apex Geoscience, March 2011).

In 2011, Apex Geoscience conducted a geotechnical investigation of the Landfill Stormwater Runoff Pond. The report recommended raising the elevation of the Landfill Stormwater Runoff Pond embankments to an elevation of 302 feet amsl using soils compacted to at least 95% standard proctor density, and an emergency spillway with a crest elevation of 298 feet amsl (Apex Geoscience, April 2011). These recommendations were implemented during subsequent Landfill Stormwater Runoff Pond construction activities.

In 2012, Apex Geoscience conducted a geotechnical investigation for Landfill expansion activities at the western portion of the Landfill where surface lignite mining operations had previously been conducted to a depth of 50 to 100 feet using a dragline, and the spoils (reclaimed soil) were returned to the excavation. The report concluded the Landfill embankments would be stable with side slopes of 3:1 (Apex Geoscience, 2013).

In 2015, Auckland Consulting further expanded the groundwater monitoring well system at the Plant, including installation of monitoring wells AD-30 through AD-35 (Auckland Consulting, 2016).

In 2016, Auckland Consulting conducted a geotechnical evaluation of the 2018 Landfill Lateral Expansion area, including installation of eight soil borings (B1 through B8) with total depths ranging from 40 to 62 feet below ground level (bgl). Soil boring locations are shown on **Figure 11**, and copies of the soil boring logs are provided in **Appendix C.** Based on the results of the 2016 geotechnical evaluation, including slope stability modeling and settlement analysis, Auckland Consulting concluded the 2018 Landfill Lateral Expansion will be structurally stable with maximum side slopes of 3:1 to a maximum disposal height of 120 feet (Auckland, November 2016).



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In 2018, Auckland Consulting installed seven piezometers (PZ-1 through PZ-7) within the 2018 Landfill Lateral Expansion area to obtain detailed depth to groundwater and groundwater flow direction data prior to construction of the Lateral Expansion. The piezometers were completed in the uppermost water-bearing unit with total depths ranging from 14 to 20 feet bgl. Piezometer locations are shown on **Figure 11**, and piezometer completion data is provided in **Appendix C.**

2.4 Hydrogeologic Setting

2.4.1 General

The site area is located within the West Gulf Coastal Plain. Cretaceous formations crop out in belts that extend in a northeasterly direction parallel to the Gulf of Mexico, and dip gently southeast. The central and northern portions of the Plant are located on the outcrop of the Eocene-age Recklaw Formation. The Recklaw Formation consists predominantly of clay and fine grained sand, and attains a maximum thickness of approximately 100 feet (Broom, 1966).

The Recklaw Formation is underlain by the Eocene-age Carrizo Sand, which outcrops in the topographically low southern portion of the Site in the area of the Landfill Stormwater Runoff Pond. The Carrizo Sand consists of fine to medium grained sand interbedded with silt and clay, and attains a thickness of up to approximately 100 feet in Harrison county, Texas (Broom, 1966). As shown on Geologic Cross Sections C-C' (**Figure 6**) and D-D' (**Figure 7**), a thick sand stratum is located below and adjacent to the Landfill between an elevation of approximately 270 feet and 330 feet amsl. This sand stratum likely corresponds to the Carrizo Sand based on geologic maps of the Site area (Broom, 1966; Flawn, 1965).

The Carrizo Sand is underlain by the Eocene-age Wilcox Formation, which outcrops in topographically low areas near the Sabine River to the south and southeast of the Plant (Flawn, 1965). The Wilcox Formation consists of interbedded sand and clay with seams of lignite and attains a thickness of approximately 700 feet (Broom, 1966). As shown on Geologic Cross Section D-D' (**Figure 7**), a lignite seam was encountered below an elevation of approximately 270 feet amsl during drilling of monitoring well AD-24 at the south end of the Site. This lignite seam likely corresponds to the top of the Wilcox Formation based on geologic maps of the Site area (Broom, 1966; Flawn, 1965).

These features are further illustrated on five lines of cross section that were prepared through the existing Landfill Area, with three lines trending from west to east (A-A'; B-B'; C-C'), and the other two lines trending from north to south (D-D'; E-E'). The cross section location map is included as **Figure 3** and the lines of cross section are included as **Figure 4** (A-A') through **Figure 8** (E-E').



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2.4.2 2018 Landfill Lateral Expansion Area

Based on the hydrogeologic data obtained from soil borings, monitoring wells, and piezometers installed in the 2018 Landfill Lateral Expansion area, three lines of cross section were prepared through the 2018 Landfill Lateral Expansion area. The cross section location map is included as **Figure 11** and the lines of cross section are included as **Figure 12** (F-F'), **Figure 13** (G-G'), and **Figure 14** (H-H').

As shown on **Figures 12** through **14**, a reddish-brown to gray clayey and silty sand stratum is located below the Lateral Expansion area between an elevation of approximately 280 feet and 330 feet amsl. This sand stratum likely corresponds to the Carrizo Sand based on geologic maps of the Site area (Broom, 1966; Flawn, 1965).

As shown on **Figures 12** through **14**, a clay stratum with an average thickness of approximately 10 feet is located below the sand stratum, and a lignite seam is present below the clay stratum at an elevation of approximately 270 feet amsl. As discussed above in Section 2.4.1, this lignite seam likely corresponds to the top of the Wilcox Formation based on geologic maps of the Site area (Broom, 1966; Flawn, 1965).

2.4.3 Climate and Water Budget

Average temperatures in Harrison County, Texas range from 47.1° Fahrenheit (F) in January to 83.8°F in July, and the mean annual growing season is 238 days. Average annual precipitation (including liquid water equivalent from snowfall) is approximately 47 inches (Broom, 1966).

2.4.4 Regional and Local Geologic Setting

The central and northern portions of the Plant are located on the outcrop of the Eocene-age Recklaw Formation. The Recklaw Formation is underlain by the Eocene-age Carrizo Sand, which outcrops in the topographically low southern end of the Plant where the existing Landfill, 2018 Landfill Lateral Expansion, and Landfill Stormwater Runoff Pond are located (Broom, 1966; Flawn, 1965).

Detailed regional geologic characterization can be found in several published reports including Texas Water Development Report 27 "Ground-Water Resources of Harrison County, Texas" (Broom, 1966), The University of Texas at Austin Bureau of Economic Geology "Geologic Atlas of Texas – Tyler Sheet" (Flawn, 1965), and U.S. Geological Survey Open-File Report 88-450K "Petroleum Geology and the Distribution of Conventional Crude Oil, Natural Gas, and Natural Gas Liquids, East Texas Basin" (USGS, 1988).

Detailed regional and site geologic characterization can also be found in the 2010 ETTL report entitled "Geotechnical Investigation, Pirkey Power Station, Existing Ash, Surge, Lignite and Limestone Runoff, and Landfill Stormwater Ponds Embankment Investigation, Hallsville, Texas" (ETTL, 2010).



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2.4.5 Surface Water and Surface Water Groundwater Interactions

Figures 9 and **10** are potentiometric surface maps based on January 2016 and August 2017 water level data, respectively, for the uppermost water bearing unit at the Site, and water level elevations in the Site monitoring wells are summarized on **Table 1**. As shown on **Figures 9** and **10**, shallow groundwater flow direction in the Landfill area is southwesterly at an average hydraulic gradient of approximately 0.01 foot per foot.

The Landfill is located approximately 400 feet west of Brandy Branch Reservoir, which was dammed during Plant construction in the 1980's. The normal pool level of Brandy Branch Reservoir is approximately 340 feet amsl. As shown on **Figures 9** and **10**, shallow groundwater flow direction at the Site generally follows surface topography to the west and southwest toward Hatley Creek, which is located in a topographically low area approximately one mile west of the Site. Therefore shallow groundwater in the Landfill area does not discharge into Brandy Branch Reservoir. Brandy Branch Reservoir likely recharges the uppermost water bearing unit in the southern portion of the Site, where the pool level in the Reservoir (340 feet amsl) is higher than water level elevations in monitoring wells located southwest (downslope) of the Reservoir.

Figure 15 is a current potentiometric surface map for the 2018 Landfill Lateral Expansion area, and water level elevations for the 2018 Landfill Lateral Expansion area piezometers are summarized on **Table 2**. As shown on **Figure 15**, shallow groundwater flow direction in the 2018 Landfill Lateral Expansion area is westerly toward the Landfill Stormwater Runoff Pond at a hydraulic gradient of approximately 0.02 foot per foot. Water level elevations in the 2018 Landfill Lateral Expansion area piezometers are higher than the surface water elevation of the Landfill Stormwater Runoff Pond (approximately 298 feet amsl), which indicates shallow groundwater in the 2018 Landfill Lateral Expansion area discharges into the Landfill Stormwater Runoff Pond.

2.4.6 Water Users

A water well inventory conducted by Banks Information Solutions showed 12 water wells had been drilled within a ½-mile radius of the Site (Banks, 2015). The nearest water well was reportedly drilled directly east of the Landfill in 2004 by Bennett Drilling for use as a rig supply well. The water well was screened from 330 to 426 feet below ground surface, therefore this water well is completed in a deeper water bearing unit relative to the uppermost water-bearing unit at the Site.

The second closest water well was reportedly drilled directly south of the Landfill by Amoco Production Company in 1991 for use as an oil field rig supply well. The water well was screened from 163 to 243 feet below ground surface, therefore this water well is completed in a deeper water bearing unit relative to the uppermost water-bearing unit at the Site.

The third closest water well was reportedly drilled approximately 200 feet southwest of the Landfill by Matador Operating in 2000 for use as an industrial well. The water well



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was screened from 340 to 420 feet below ground surface, therefore this water well is completed in a deeper water bearing unit relative to the uppermost water-bearing unit at the Site.

All of the water wells identified within a $\frac{1}{2}$ -mile radius of the Site were drilled to total depths of 160 feet or deeper except one water well (Well ID: 35-37-4E) that was drilled to a total depth of 55 feet in 1982. This water well was completed with concrete tile from the surface to total depth, and is located approximately $\frac{1}{4}$ -mile east (up gradient) of the Pirkey Power Plant.



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3. Isolation from the Uppermost Aquifer

CCR Rule 40 CFR Part 257.60 requires that the base of new and existing CCR surface impoundments be constructed such that the base of the unit is no less than 5 ft above the top of the uppermost aquifer, or that if the base is within 5 ft of the uppermost aquifer, that there will not be hydraulic connection between the base of the unit and the uppermost aquifer.

3.1 Uppermost Aquifer and Piezometric Analysis

3.1.1 Piezometric Analysis

3.1.1.1 Horizontal and Vertical Position Relative to CCR Unit

Geologic data from soil borings, piezometers, and monitoring wells installed in the 2018 Landfill Lateral Expansion area show the uppermost aquifer is a reddish-brown to gray clayey and silty sand stratum located between an elevation of approximately 280 feet and 330 feet amsl. As shown on **Figure 15** and **Table 2**, this sand stratum is saturated at an elevation ranging from approximately 298 feet amsl (PZ-1) in the topographically low northwest portion of the Lateral Expansion area to approximately 320 feet amsl (AD-23) in the topographically high southeast portion of the Lateral Expansion area.

As discussed above in Section 2.2.3, the 2018 Landfill Lateral Expansion design includes a liner system which will consist of a 60-mil HDPE top liner underlain by a 2foot-thick compacted clay bottom liner. Prior to installation of the liner system. approximately 10 to 15 feet of cut into the existing soils will occur along the topographically higher southern portion of the Lateral Expansion, and emplacement of these soils (clayey sands, silty clayey sand and/or lean clays) as fill materials below the liner system in the topographically lower central and northern portions of the Lateral Expansion. The fill materials emplaced in the topographically lower portions of the Lateral Expansion will extend in height at least five feet above the water table, thereby providing the required separation distance of at least five feet between the base of the CCR unit and the top of the uppermost aquifer. This separation distance is illustrated on Figure 16 which shows the elevation of the HDPE liner at the base of the Lateral Expansion. The elevation of the HDPE liner in the topographically low northwest portion of the Lateral Expansion will range from approximately 311 to 314 feet amsl, which is more than five feet higher than the elevation of the piezometric surface in this area (approximately 298 to 304 feet amsl).

3.1.1.2 Overall Flow Conditions

Groundwater is recharged from regional precipitation infiltration. The uppermost water bearing unit (clayey and silty sand) is expected to have a hydraulic conductivity of approximately 1 x 10⁻⁴ cm/sec (Fetter, 1980). Based on the hydraulic conductivity and saturated thickness (approximately 20 to 30 feet), the yield of the uppermost water-



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bearing unit in the Lateral Expansion area is anticipated to exceed the TCEQ non-useable (Class 3) limit of 150 gallons per day (TCEQ, 2010).

Current groundwater elevations in the Lateral Expansion area are summarized on **Table 2**, and a recent piezometric surface map is depicted on **Figure 15**. The piezometric surface map is based on water level elevations measured on June 22, 2018 when water levels were high due to a recent rainfall event. The June 22, 2018 water level elevations are more than 5 feet below the planned Lateral Expansion HDPE liner elevation, indicating the base of the CCR unit is more than 5 feet above the uppermost aquifer.

3.1.2 Uppermost Aquifer

3.1.2.1 CCR Rule Definition

The CCR rule definitions for an aquifer and the uppermost aquifer as specified in 40 CFR 257.53 indicates an aquifer is a geologic formation capable of yielding usable quantities of groundwater to wells or springs while an uppermost aquifer is defined as the geologic formation nearest the natural ground surface that is an aquifer, as well as lower aquifers, that are hydraulically interconnected with this aquifer within the facility's property boundary. Upper limit is measured at a point nearest to the natural groundwater surface to which the aquifer rises during the wet season.

3.1.2.2 Common Definitions

An aquifer is commonly defined as a geologic unit that stores and transmits water (readily or at sufficient flow rates) to supply wells and springs (USGS, 2015; Fetter, 2001). The uppermost aquifer is considered the first encountered aquifer nearest to the CCR unit.

3.1.3 Identified Onsite Hydrostratigraphic Unit

The identified Site hydrostratigraphic unit in the area of the 2018 Landfill Lateral Expansion area is the reddish-brown to gray clayey and silty sand stratum located between an elevation of approximately 280 feet and 330 feet amsl.

3.2 Compliance with Isolation Distance

The uppermost water-bearing unit underlying the 2018 Landfill Lateral Expansion meets the regulatory definition of an aquifer. As shown on **Figure 16**, the HDPE liner below the base of the CCR unit will be more than 5 feet above this aquifer. Therefore, this CCR Unit complies with the requirement for placement above the uppermost aquifer. Also, underground lateral perforated HDPE underdrain pipes will be installed in permeable bottom ash materials approximately seven feet below the 60-mil HDPE liner of the 2018 Landfill Lateral Expansion. These undrains will collect shallow groundwater when the water table is seasonally high, and gravity drain the groundwater into the Landfill Stormwater Runoff Pond, which will maintain the separation distance of at least 5 feet below the base of the CCR unit.



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4. Wetlands

CCR Rule 40 CFR Part 257.61 requires that new CCR Landfill Lateral Expansions not be located in wetlands.

4.1 Local Wetlands

Based on review of available published information, including the National Hydrography Dataset (NHD) and National Wetland Inventory (NWI) maps, there are no jurisdictional wetlands in the 2018 Landfill Lateral Expansion area (USFWS, 2018; USGS, 2018). The topographically low northwest portion of the Lateral Expansion area has no connections to jurisdictional waters of the U.S. (WOTUS) and is non-jurisdictional. **Figure 17** is a map showing wetlands locations in the CCR unit area.

4.2 Compliance with Wetland Restrictions

Based on review of available information, the 2018 Landfill Lateral Expansion area does not contain wetlands. Therefore, the 2018 Landfill Lateral Expansion complies with the requirement for not being located in a wetland.



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5. Fault Areas

CCR Rule 40 CFR Part 257.62 requires that new CCR Landfill Lateral Expansions must not be located within 200 feet of the outermost damage zone of a fault that has had displacement in Holocene time unless the owner or operator demonstrates that an alternate setback will prevent damage to the structural integrity of the CCR unit.

5.1 Description of Regional Geologic Structural Features

Regional geologic publications were reviewed to determine structural features for the Site. A regional fault map is provided on **Figure 18**. The U.S. Geological Survey Open File Report 88-450K shows the Site is located within the East Texas Basin, with faulting north of the basin (Talco Fault Zone) and south of the basin (Elkhart-Mt. Enterprise Fault Zone). No faulting was identified in the Site area (USGS, 1988). Texas Water Development Board Report 27, and the University of Texas at Austin Bureau of Economic Geology Geologic Atlas of Texas – Tyler Sheet show no faulting at the Site (Broom, 1966; Flawn, 1965).

A previous evaluation of geologic structural features at the Site was conducted by ETTL, and no evidence of faulting was identified (ETTL, 2010).

5.2 Compliance with Fault Area Restrictions

A review of available geologic reports and maps has indicated that the Site is not located near any faults with displacement in the Holocene. Therefore, the Landfill Lateral Expansion complies with the requirement for not being located within 200 feet of the outermost damage zone of a fault that has had displacement in Holocene time.



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6. Seismic Impact Zone

CCR Rule 40 CFR Part 257.63 requires that new CCR Landfill Lateral Expansions must not be located within a seismic impact zone unless the owner or operator demonstrates that all structural components of the CCR unit are designed to withstand the maximum horizontal acceleration in lithified earth material for the site.

6.1 Definition of Seismic Impact Zone

CCR Rule 40 CFR Part 257.53 defines a seismic impact zone as an area having a 2% or greater probability that the maximum horizontal acceleration expressed as a percentage of the earth's gravitational pull (g) will exceed 0.10 g in 50 years.

6.2 Compliance with Seismic Impact Zone Restriction

Figure 19 presents the seismic hazard map for Texas, as published by the USGS. As shown on **Figure 19**, the Site falls within the zone having a maximum horizontal acceleration of 0.04 to 0.06 g. Therefore, the Landfill Lateral Expansion complies with the requirement for not being located in a seismic impact zone.



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7. Unstable Areas

CCR Rule 40 CFR Part 257.64 requires that existing and new CCR landfills and lateral expansions must not be located within an unstable area unless the owner or operator demonstrates that the design of the unit will ensure the integrity of the structural components of the unit.

7.1 Definition of Unstable Area and local Conditions

7.1.1 CCR Rule Definition

CCR Rule 40 CFR Part 257.53 defines an unstable area as a location that is susceptible to natural or human-induced events or forces capable of impairing the integrity of the CCR unit. These may include poor foundation conditions, areas susceptible to mass movements (landslides), and karst terrains.

7.1.2 Poor Foundation Soils

ETTL conducted a geotechnical investigation and foundation settlement evaluation of the Landfill foundation in 2005. The investigation included evaluation of the western portion of the Landfill where lignite mining operations previously occurred. The ETTL evaluation concluded the predicted Landfill settlement would not exceed approximately 4.3 feet and would not adversely affect the performance of the Landfill liner or leachate collection system, and the Landfill excavation, interim fill, and final cover would be stable on slopes of 3:1 or 4:1 (ETTL, 2005).

Apex Geoscience conducted a geotechnical investigation and foundation settlement evaluation of the Landfill in 2012, including advancement of two CPT borings in the western portion of the Landfill where lignite mining operations had previously occurred. The report predicted estimated settlement of 39.07 inches (3.26 feet) and concluded the Landfill would be stable with side slopes of 3:1 (Apex Geoscience, 2013).

Auckland Consulting conducted geotechnical evaluation of the 2018 Landfill Lateral Expansion area in 2016, including advancement of eight soil borings (B1 through B8) through the Lateral Expansion area, and a settlement analysis based on CCR material loading to the maximum design waste height of 120 feet. The report predicted estimated settlement of up to 43.96 inches (3.66 feet) and concluded the Lateral Expansion would be stable with side slopes of 3:1 (horizontal to vertical) and bench widths of approximately 16 feet (Auckland Consulting, 2016). The report also stated the fill soils to be emplaced below the liner system in the topographically lower central and northern portions of the Lateral Expansion will be placed in loose lifts not exceeding 8 inches and compacted to a minimum 95% of the maximum density as determined by Standard Proctor test ASTM D 698 (Auckland Consulting, 2016).



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7.1.3 Mass Movements

Geotechnical evaluations of the Landfill by ETTL in 2005, Apex Geoscience in 2012, and Auckland Consulting in 2016 concluded the Landfill would not be subject to mass movements that could impair the integrity of the Landfill based on side slopes of 3:1 (ETTL, 2005; Apex Geoscience, 2013; Auckland Consulting, 2016).

7.1.4 Karst

The site area is located on the outcrop of unconsolidated Cretaceous Formations consisting predominantly of sand and clay (Broom, 1966; Flawn, 1965). The Landfill is not located in a karst area.

7.1.5 Subsurface Mining

Naturally occurring lignite is present in portions of the Site area, and a naturally occurring lignite seam was identified at an elevation of approximately 270 feet amsl at monitoring well AD-24 as shown on Geologic Cross Section D-D' (**Figure 7**). The Texas Water Development Board Ground-Water Resources Report for Harrison County, Texas, states that the Wilcox Formation, which underlies the Carrizo Sand, contains lignite (Broom, 1966).

Lignite mining operations using a drag line had occurred in the western portion of the Landfill prior to Landfill construction in this area (VFL Technology Corporation, 1984). The drag line mining method is a surface mining method. A geotechnical evaluation of the previously mined western portion of the Landfill was conducted by ETTL in 2005. The ETTL report indicated the western portion of the Landfill had been mined in the past to a depth of 50 to 100 feet using a dragline, and the spoils (reclaimed soil) were returned to the excavation. The geotechnical evaluation included installation of 30 soil borings, ten CPT borings, and geotechnical testing of soil samples. The ETTL report concluded the Landfill would be stable based on the Landfill liner system (compacted clay or GCL overlain by HDPE or VLDPE), leachate drainage system (lateral underdrain consisting of permeable bottom ash), and Landfill side slopes no steeper than 3:1 (ETTL, 2005).

No subsurface mining is known to exist below the 2018 Landfill Lateral Expansion area.

7.2 Compliance with Unstable Areas Restriction

Based on our review of available information, including the 2005 geotechnical evaluation of the Landfill by ETTL, the 2012 geotechnical evaluation of the Landfill by Apex Geoscience, and the 2016 geotechnical evaluation of the Lateral Expansion Area by Auckland Consulting, this CCR unit complies with the requirement for not being located in an unstable area.



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8. Summary, Conclusions, and PE Certification

I, Kenneth J. Brandner, certify that this report was prepared under my direction and supervision, and that the information contained herein is true and accurate to the best of my knowledge. Based on my experience and knowledge of the site, as well as the evaluations discussed within this report, the H.W. Pirkey Power Plant 2018 Landfill Lateral Expansion complies with the requirements of the location restrictions sections of 40 CFR 257 Subpart D that apply to landfills and therefore the CCR unit is not located in a restricted location.

Renneth J. Brandner
Printed Name of Registered Professional Engineer

Signature

69586

Registration No.

lexas

Registration State

10-25-18

Date



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Table 1 **Water Level Data AEP Pirkey Power Plant - CCR Storage Areas** Hallsville, Harrison County, Texas

			Ground	Top of	Borehole	Date	Screen	Well	Top of	Screen ^(b)	Bottom o	of Screen ^(b)	4/13/2011	12/15/2011	6/20/2012	1/23/2013	7/7/2013	1/22/2014	7/9/2014	1/28/2015	1/20/2016	1/12/2017	3/1/2017	4/11/2017	8/24/2017
			Surface	Casing	depth	Installed	Material	diameter	Depth	Elevation	Depth	Elevation	GW Elev.	GW Elev.	GW Elev.	GW Elev.	GW Elev.	GW Elev.	GW Elev.	GW Elev.	GW Elev.	GW Elev.	GW Elev.	GW Elev.	GW Elev.
Well ID	Latitude	Longitude	Elevation ^(a)	Elevation ^(a)	ft. bls			inches	ft. bls	ft. msl	ft. bls	ft. msl	ft. msl	ft. msl	ft. msl	ft. msl	ft. msl	ft. msl	ft. msl	ft. msl	ft. msl	ft. msl	ft. msl	ft. msl	ft. msl
Monitoring Wells																									
MW-2/AD-2	32° 27' 54.753"	94° 29' 25.282"	341.25	344.04	40	10/7/83	Sch. 40 PVC	4	20	321.25	40	301.25	326.90	327.12	327.17	327.26	326.62	327.70	327.19	328.62	328.55	327.65	327.96	329.09	327.63
MW-3/AD-3	32° 28' 6.829"	94° 29' 21.498"	372.76	375.30	57	11/4/83	Sch. 40 PVC	4	37	335.76	57	315.76	342.95	341.59	343.70	341.10	343.27	341.42	343.96	345.01	347.03	344.19	345.53	345.53	343.49
MW-4/AD-4	32° 27' 59.247"	94° 29' 4.692"	363.69	366.79	46	10/10/83	Sch. 40 PVC	4	26	337.69	46	317.69	351.45	351.24	352.44	354.42	349.22	355.58	353.33	359.00	359.16	353.27	355.38	356.62	353.58
MW-7/AD-7	32° 27' 43.611"	94° 29' 15.611"	359.61	362.79	40	10/3/83	Sch. 40 PVC	4	20	339.61	40	319.61	344.34	343.75	344.15	344.90	343.35	346.61	346.23	349.17	349.31	347.04	347.96	347.87	347.40
MW-8/AD-8	32° 27' 25.095"	94° 29' 14.925"	356.92	359.84	35	10/4/83	Sch. 40 PVC	4	20	336.92	35	321.92	341.65	340.29	341.65	340.72	341.25	341.67	343.36	344.03	347.21	345.74	346.00	345.81	346.31
MW-10/AD-10	32° 27' 52.446"	94° 29' 16.545"	359.48	362.21	40	10/10/83	Sch. 40 PVC	4	20	339.48	40	319.48	342.03	341.90	342.19	341.41	339.85	342.27	342.22	344.39	343.97				
MW-12/AD-12	32° 27' 51.702"	94° 29' 3.238"	378.84	381.99	51	1/30/86	Sch. 40 PVC	4	31	347.84	51	327.84	358.95	357.99	359.33	368.07	357.41	369.97	367.04	372.75	371.05	365.11	368.79	372.97	367.68
MW-13/AD-13	32° 27' 46.002"	94° 29' 5.71"	361.98	364.76	40.5	2/23/88	Sch. 40 PVC	4	30.5	331.48	40.5	321.48	349.46	348.91	349.52	350.81	348.61	351.97	351.29	354.47	354.15	352.01	352.81	352.68	352.62
AD-16	32° 27' 40.871"	94° 29' 38.637"	356.81	360.05	35	12/30/10	Sch. 40 PVC	2	15.0	341.81	35.0	321.81	338.08	335.50	337.58	335.43	336.67	339.53	340.84	343.34	347.68	343.09	344.54	344.69	342.71
AD-17	32° 28' 2.315"	94° 29' 39.45"	342.65	346.09	30	12/30/10	Sch. 40 PVC	2	10.0	332.65	30.0	312.65	322.66	322.29	323.31	323.51	323.06	325.19	324.15	328.42	326.78	324.70	326.27	326.27	324.18
AD-18	32° 28' 9.245"	94° 29' 6.469"	360.48	363.42	25	1/3/11	Sch. 40 PVC	2	15.0	345.48	25.0	335.48	355.53	351.54	357.21	355.47	357.23	360.03	358.06	359.88	360.52	357.06	359.21	358.63	358.23
AD-19	32° 27' 50.512"	94° 29' 13.973"	359.50	362.82	30	12/30/10	Sch. 40 PVC	2	10.0	349.50	30.0	329.50	344.07	343.58	344.29	344.62	342.60	345.11	345.76	347.92	347.40		-		
AD-20	32° 27' 51.346"	94° 29' 21.576"	352.30	355.79	35	12/28/10	Sch. 40 PVC	2	15.0	337.30	35.0	317.30	334.50	334.63	334.69	334.78	333.38	335.38	334.87	336.88	336.07				
AD-21	32° 27' 45.403"	94° 29' 19.195"	347.23	350.72	30	12/27/10	Sch. 40 PVC	2	10.0	337.23	30.0	317.23	340.43	340.02	340.22	341.57	339.16	342.36	341.67	345.45	343.82		-		-
AD-22	32° 27' 41.349"	94° 29' 17.779"	355.57	358.51	30	12/16/10	Sch. 40 PVC	2	10.0	345.57	30.0	325.57	343.64	343.16	343.74	344.83	342.90	346.49	345.77	350.24	350.29	347.20	348.52	348.45	347.37
AD-23	32° 27' 3.384"	94° 29' 41.258"	346.72	350.10	35	12/15/10	Sch. 40 PVC	2	15.0	331.72	35.0	311.72	319.65	318.94	319.29	318.66	318.87	319.80	319.79	319.84	321.23	320.99	321.00	320.85	320.77
AD-24	32° 27' 1.455"	94° 29' 56.388"	287.68	291.14	20	12/27/10	Sch. 40 PVC	2	5.0	282.68	20.0	267.68	282.92	284.29	285.10	285.63	285.06	288.30	287.10	288.56	-				
AD-25	32° 27' 17.187"	94° 29' 58.998"	334.15	337.09	30	12/14/10	Sch. 40 PVC	2	10.0	324.15	30.0	304.15	324.51	321.90	323.14	321.94	322.15	322.56	324.24	326.42	327.00				
AD-26	32° 27' 25.426"	94° 29' 54.775"	342.41	345.25	40	12/14/10	Sch. 40 PVC	2	10.0	332.41	40.0	302.41	324.53	323.77	323.62	322.32	322.09	323.24	322.51	323.04	326.06		-		
AD-27	32° 27' 36.66"	94° 29' 47.272"	349.83	352.62	37.5	12/15/10	Sch. 40 PVC	2	17.5	332.33	37.5	312.33	325.82	324.54	326.13	325.39	325.35	326.39	327.91	329.69	330.89	330.04	331.59	331.24	330.05
AD-28	32° 27' 55.439"	94° 29' 39.418"	335.92	339.40	40	12/28/10	Sch. 40 PVC	2	15.0	320.92	35.0	300.92	319.67	319.16	319.92	320.21	319.69	320.65	320.22	322.16	321.39	320.27	320.51	320.69	320.07
AD-29	32° 28' 8.271"	94° 29' 31.939"	350.21	353.37	30	1/3/11	Sch. 40 PVC	2	10.0	340.21	30.0	320.21	334.68	333.37	334.74	337.47	336.84	338.55	335.85	340.57	338.48				
AD-30 ^(d)	32° 27' 56.49"	94° 29' 32.53"	339.04	342.02	25	12/8/15	Sch. 40 PVC	2	10.0	329.04	25.0	314.04		1							323.70	322.23	322.85	322.88	322.04
AD-31 ^(d)	32° 28' 02.48"	94° 29' 20.90"	357.75	360.75	35	12/8/15	Sch. 40 PVC	2	20.0	337.75	35.0	322.75									346.60	343.78	344.53	344.58	343.57
AD-32 ^(d)	32° 27' 56.20"	94° 29' 11.86"	357.23	359.18	33	12/11/15	Sch. 40 PVC	2	13.0	344.23	33.0	324.23									352.32	347.44	348.44	349.09	349.73
AD-33 ^(d)	32° 27' 38.70"	94° 29' 15.82"	359.30	362.37	30	12/11/15	Sch. 40 PVC	2	15.0	344.30	30.0	329.30									351.13	348.56	349.32	349.25	349.31
AD-34 ^(d)	32° 27' 10.13"	94° 29' 57.93"	304.64	307.61	25	12/11/15	Sch. 40 PVC	2	10.0	294.64	25.0	279.64									307.61	307.61	307.61	307.61	307.61
AD-35 ^(d)	32° 27' 09.64"	94° 29' 42.74"	316.01	318.95	20	12/11/15	Sch. 40 PVC	2	3.0	313.01	18.0	298.01									309.85	310.42	310.82	311.27	310.28
Piezometers ^(c)																									
W-3 (PW-3)	32° 27' 57.6"	94° 29' 31.8"	356.30	356.30	38	10/20/09	Sch. 40 PVC	2	28.0	328.30	38.0	318.30													

--- Not Measured

⁽a) Source: Apex Geoscience Inc. (March 23, 2011).(b) Screen length and screened intervals for AD-2 through AD-12 estimated from video surveillance (Apex Geoscience Inc., March 23, 2011).(c) Souce: EETL (October 2010).

⁽d) Source: Auckland Consulting LLC (January 26, 2016). Monitoring wells AD-30 through AD-35 installed during December 2015. Groundwater Elevation Source: AEP, Pirkey Monitoring Well Groundwater Elevations through January 2015.

Table 2
Piezometer Water Level Data - 2018 Landfill Lateral Expansion Area
AEP Pirkey Power Plant
Hallsville, Harrision County, Texas

Piezomenter Completion In	<u>formation</u>								
Piezometer ID:	<u>PZ1</u>	<u>PZ2</u>	<u>PZ3</u>	<u>PZ4</u>	<u>PZ5</u>	<u>PZ6</u>	<u>PZ7</u>	<u>AD-23</u>	<u>AD-35</u>
Northing	6871372.73	6871442.96	6871218.9	6871018.52	6870962.73	6870939.86	6871250.41		
Easting	3203056.63	3203345.4	3203322.02	3203009.98	3203281.7	3203544.92	3202996.36		
Screen length	10	10	10	10	10	10	10	20	15
TD (from GS)	14	14	14	14	20	20	14	37.44	18
Sand pack, top (from GS)	3	3	3	3	8	8	3		
Elev, GS								346.72	334.15
Elev, TOC	308.85	312.74	307.35	311.53	328.3	328.78	303.73	350.1	318.95
Piezometer Depth to Water	<u>Measuremen</u>	ts (feet) below	TOC						
<u>Date</u>	<u>PZ1</u>	<u>PZ2</u>	<u>PZ3</u>	<u>PZ4</u>	<u>PZ5</u>	<u>PZ6</u>	<u>PZ7</u>	<u>AD-23</u>	<u>AD-35</u>
6/20/2018	9.98	9.99	4.29	8.66	20.47	13.23	2.84		
6/21/2018	9.99	9.95	4.07	8.37	20.47	13.24	2.75	29.4	7.95
6/22/2018	9.99	9.91	3.98	8.31	20.47	13.25	2.76	29.42	7.92
6/29/2018	10.01	10.1	4.34	8.85	20.63	13.4	2.98	29.39	8.14
7/6/2018	10.02	10.23	4.45	8.92	20.75	13.52	3.21	29.43	8.23
Piezometer Potentiometric	Surface (Wate	er Table) Elevat	ions (feet AM:	SL)					
<u>Date</u>	<u>PZ1</u>	<u>PZ2</u>	<u>PZ3</u>	<u>PZ4</u>	<u>PZ5</u>	<u>PZ6</u>	<u>PZ7</u>	<u>AD-23</u>	<u>AD-35</u>
6/20/2018	298.87	302.75	303.06	302.87	307.83	315.55	300.89		
6/21/2018	298.86	302.79	303.28	303.16	307.83	315.54	300.98	320.70	311.00
6/22/2018	298.86	302.83	303.37	303.22	307.83	315.53	300.97	320.68	311.03
6/29/2018	298.84	302.64	303.01	302.68	307.67	315.38	300.75	320.71	310.81
7/6/2018	298.83	302.51	302.9	302.61	307.55	315.26	300.52	320.67	310.72
<u>Legend</u>									
GS	Ground surfac	e		TOC	Top of piezom	eter casing			
TD	Total depth			AMSL	Above mean s	ea level			

Document Path: Z:\GISPROJECTS_EN\\AEP\Pirkey Plant\MXD\Figure 1 - Site Location Map.mxd

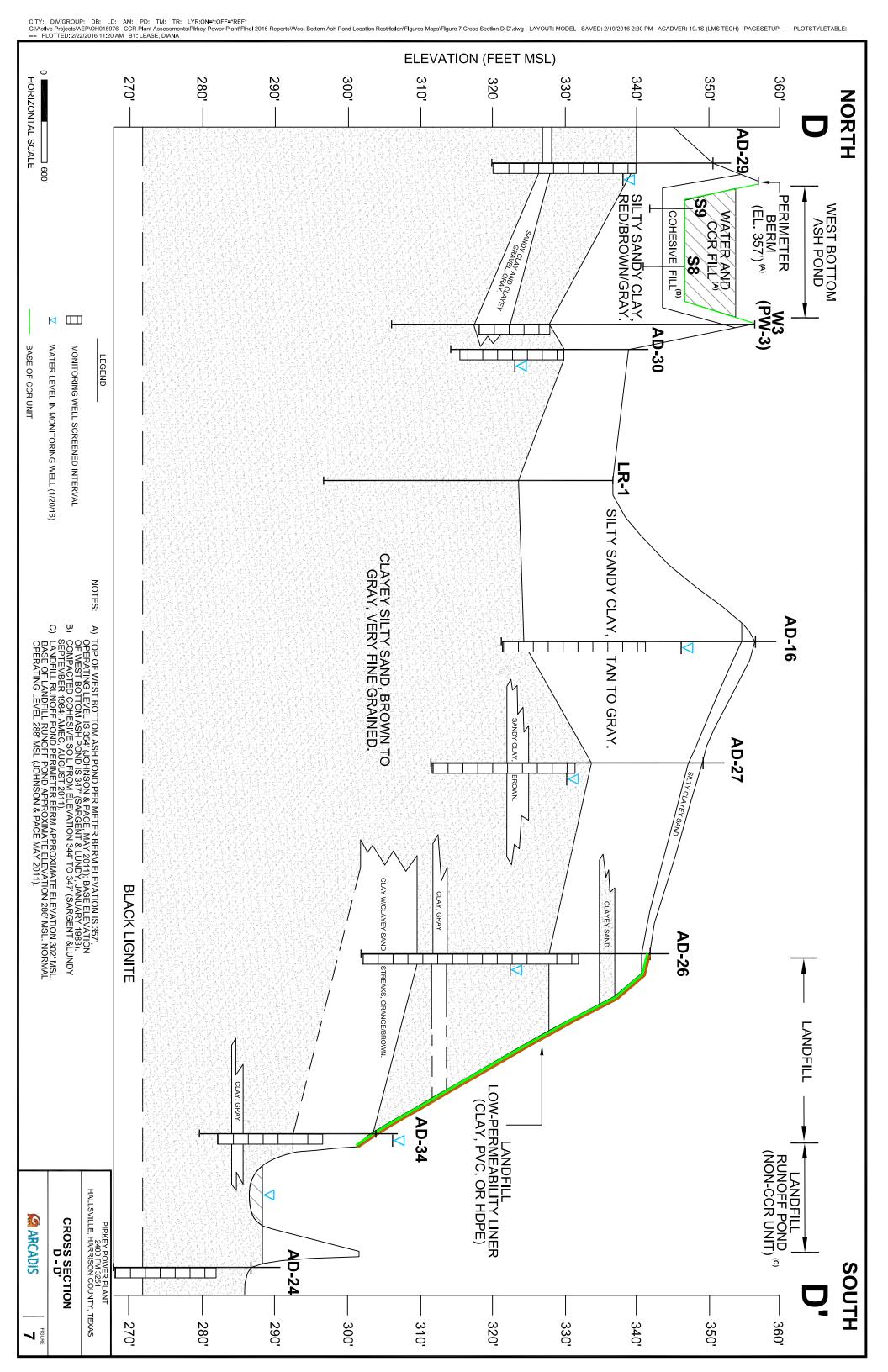
cumpart Dath: 74G1S Deciparte) ENIVAED/Dirkov Diant/MYD/Eigure 2 - Diant and CCD Hait Location Man myd

cument Path: Z:\GISProiects\ ENV\AEP\Pirkev Plant\MXD\Figure 3 - Site Layout and Well Locations.mxd

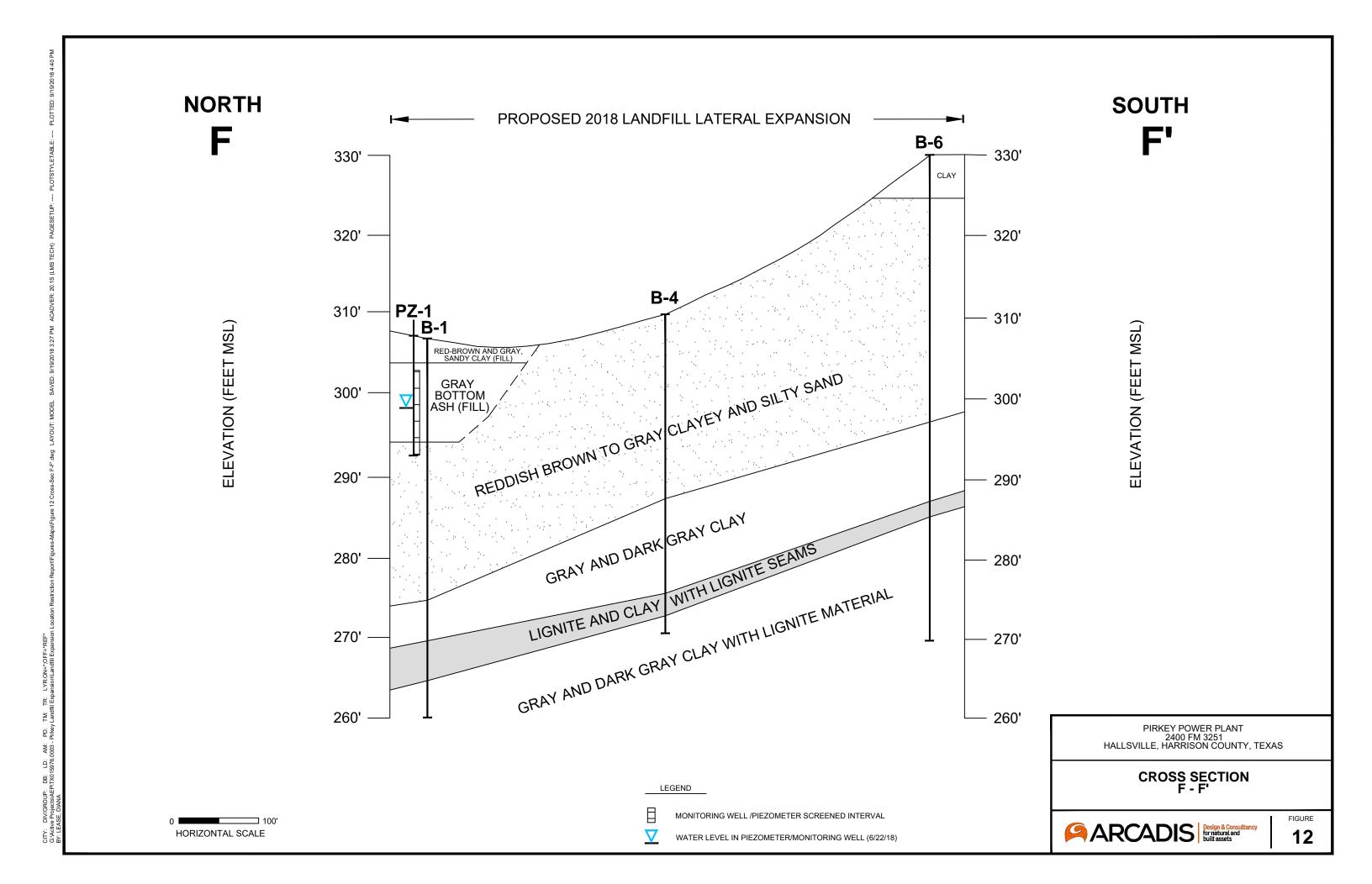
FIGURE

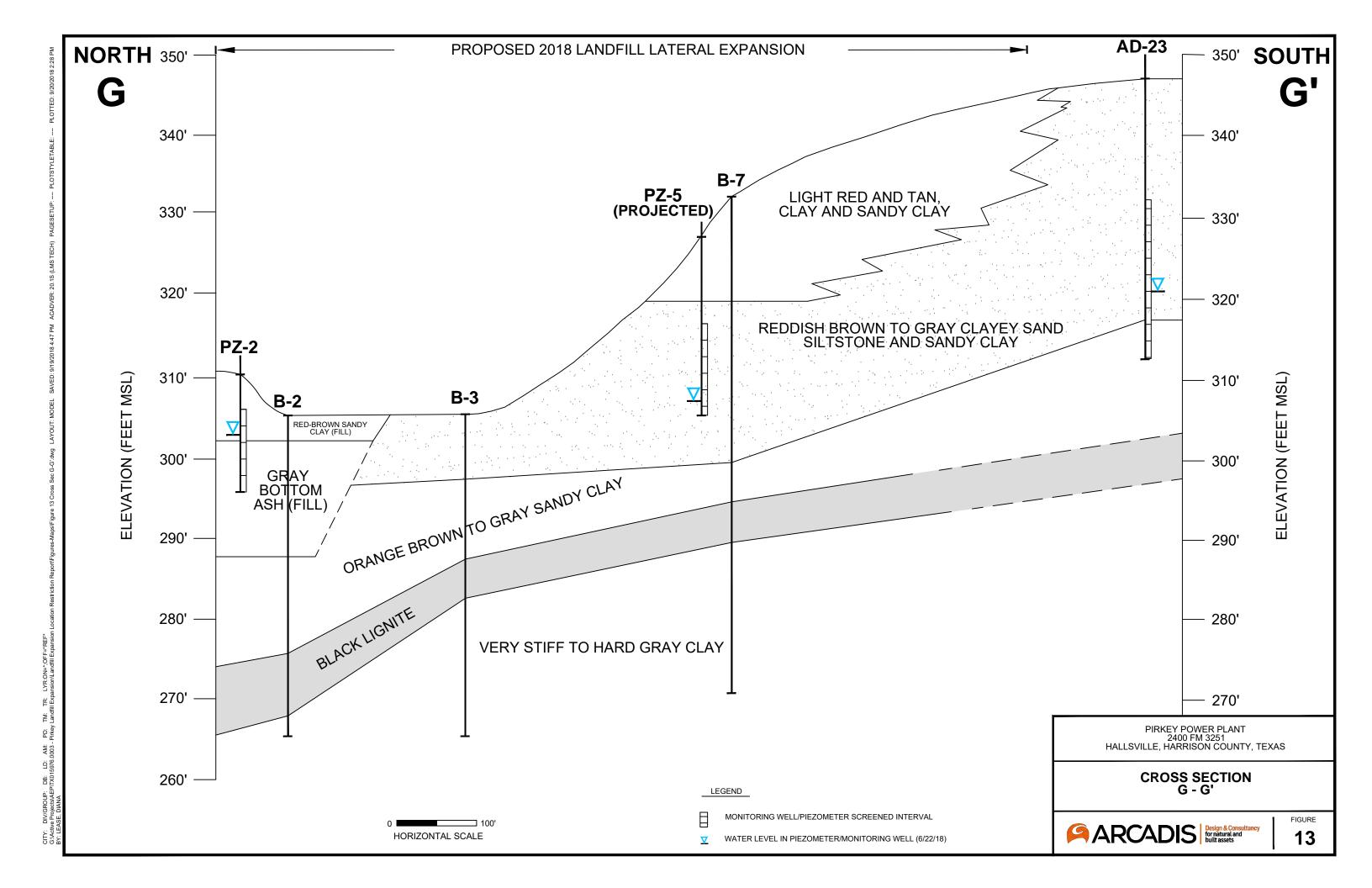
5

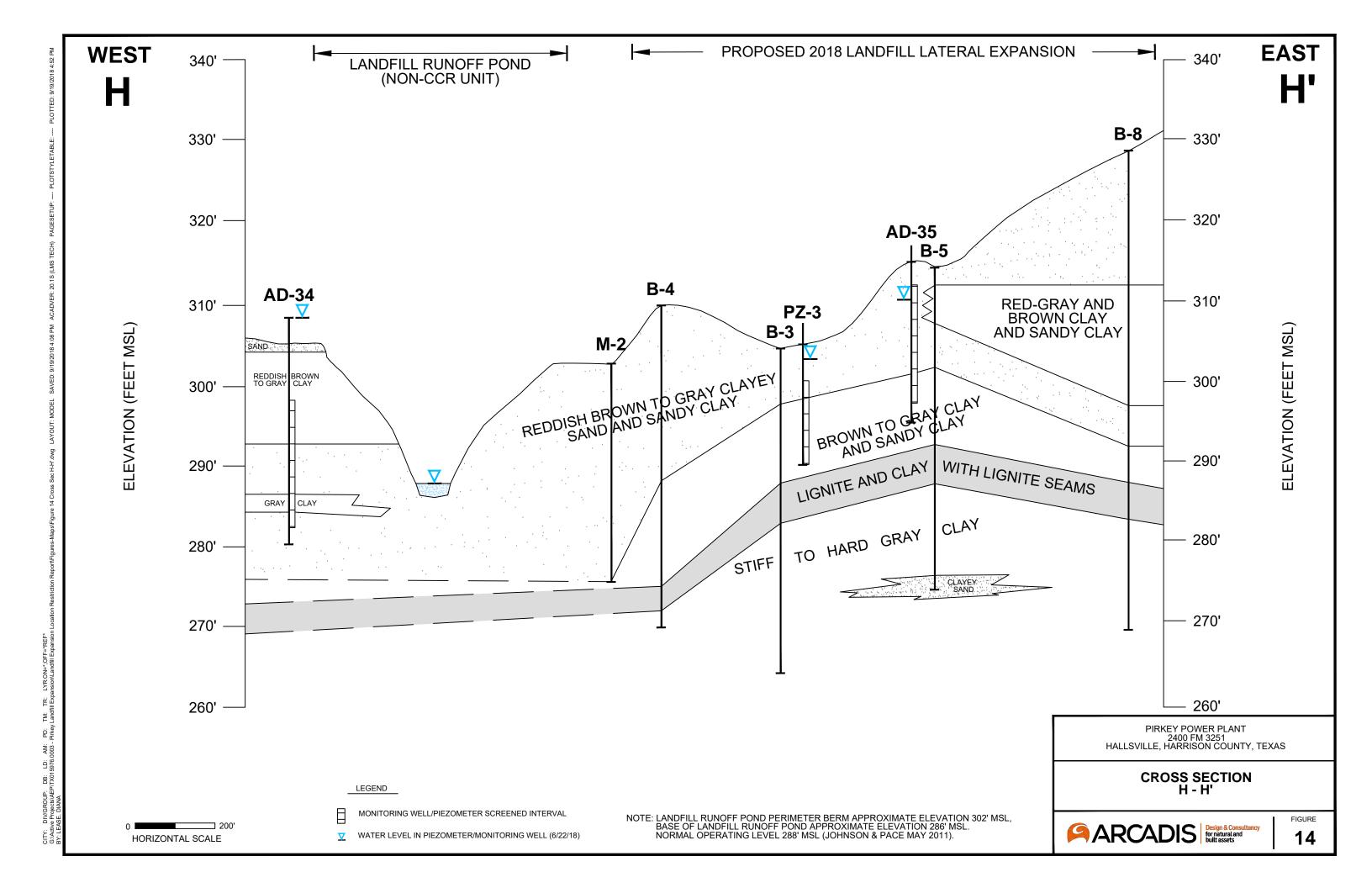
o FIGURE

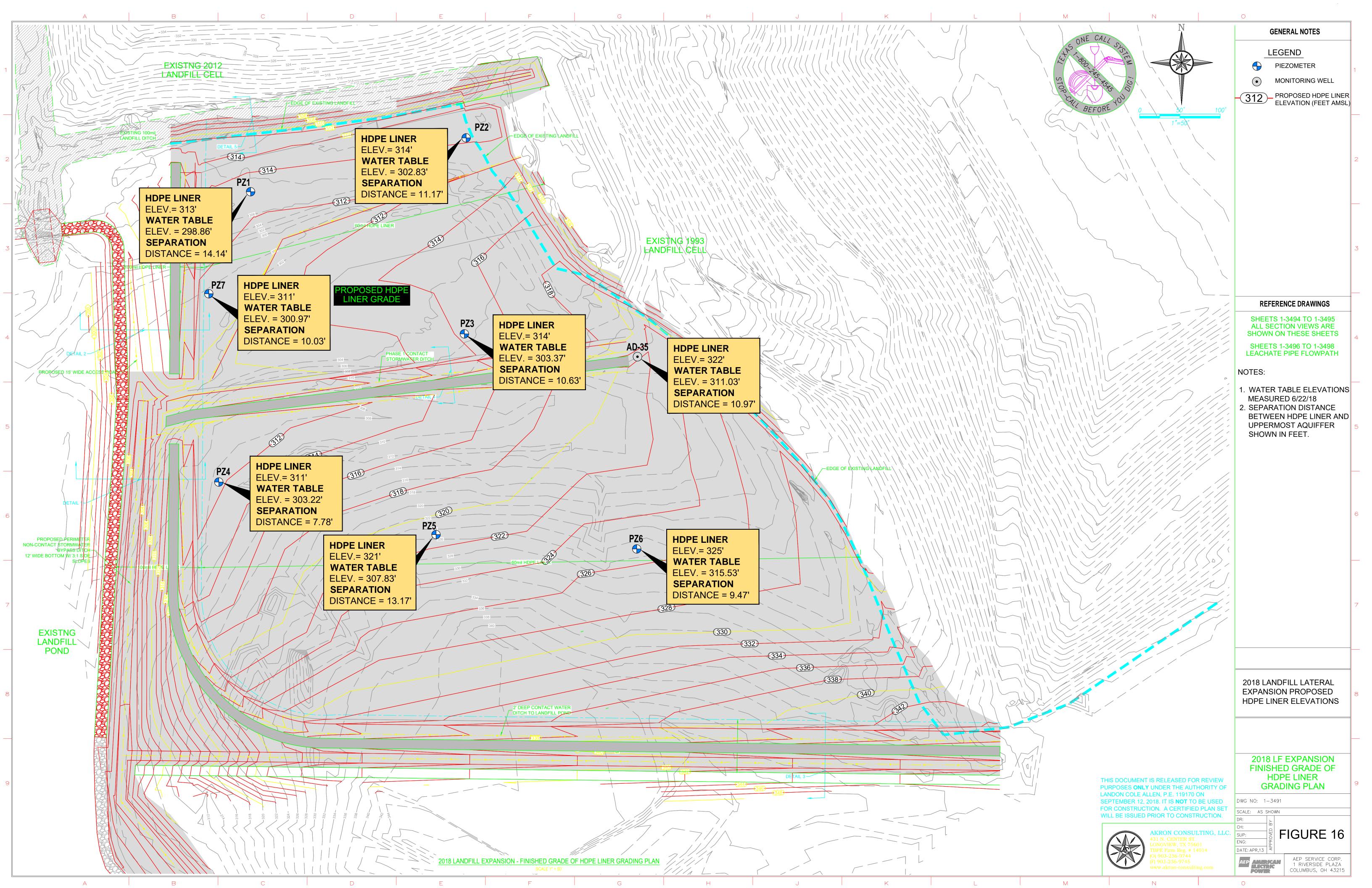


cument Path: Z.\G\SProiects\ ENV\AEP\Pirkev Plant\MXD\Figure 11 - Landfill - 2018 Cell Expansion.mxd









U.S. Fish and Wildlife Service National Wetlands Inventory

National Wetlands Inventory Map



September 13, 2018

Wetlands

Estuarine and Marine Deepwater

Estuarine and Marine Wetland

Freshwater Emergent Wetland

Freshwater Pond

Freshwater Forested/Shrub Wetland

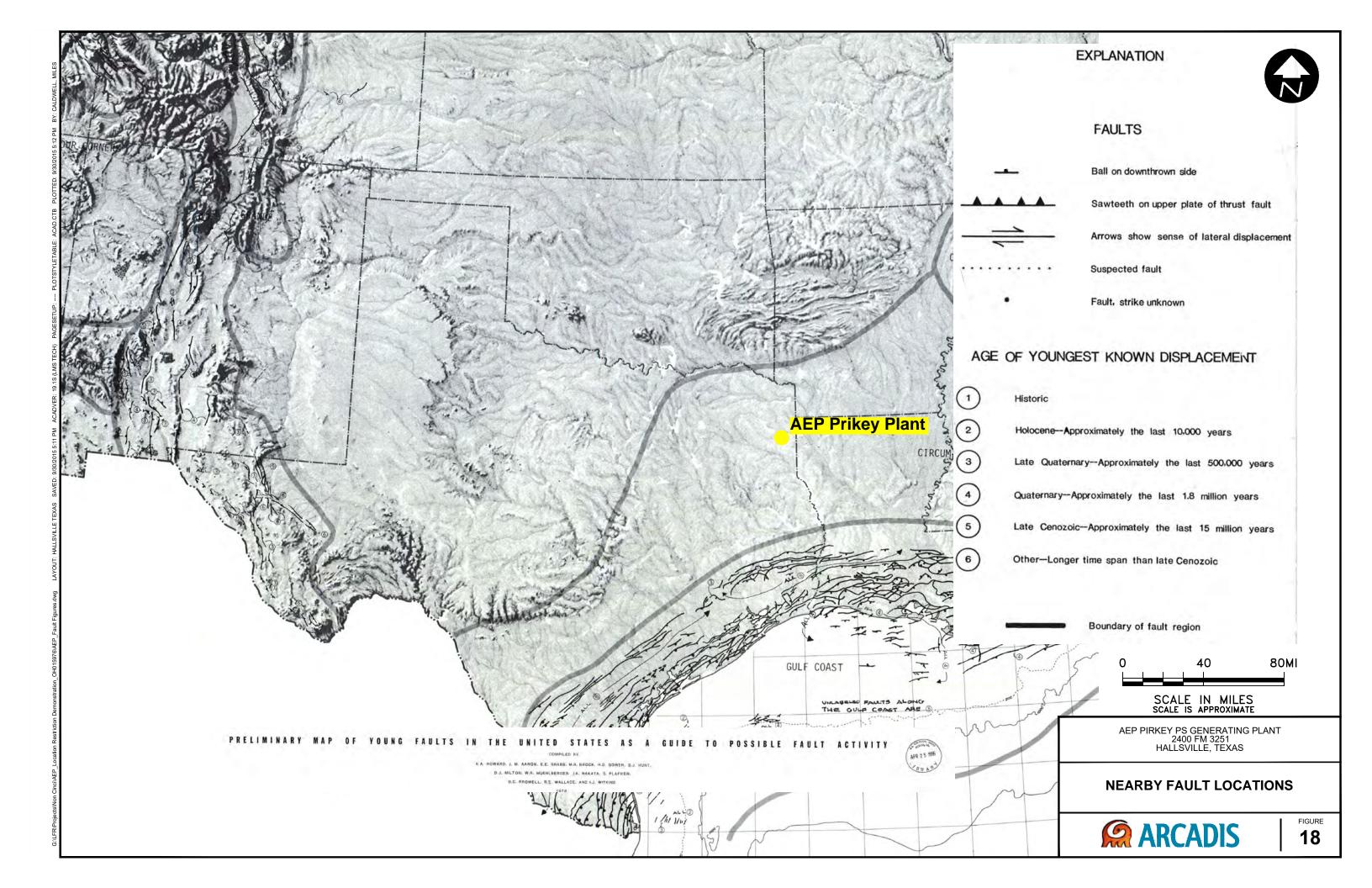
Lake

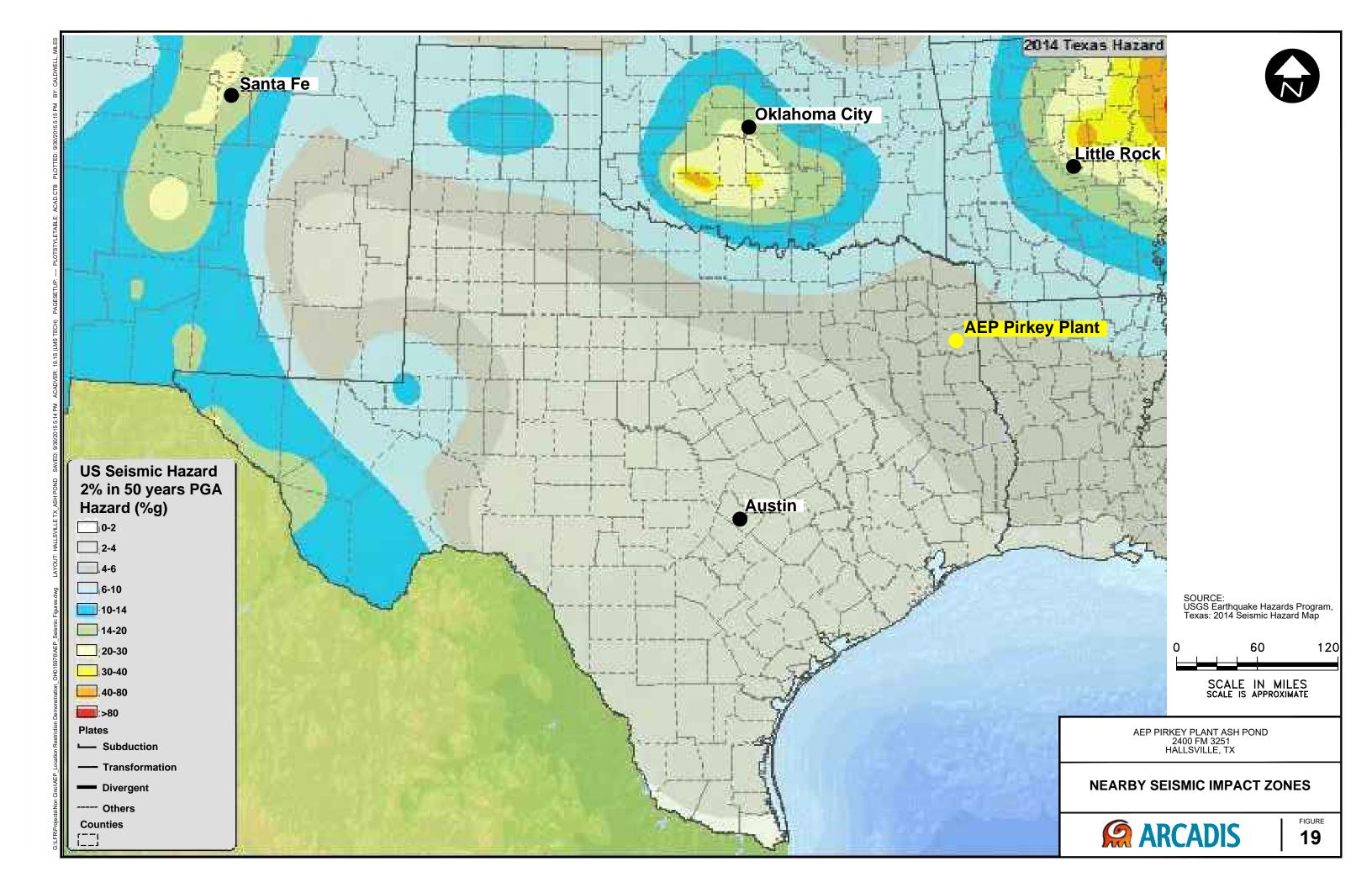
Other

Riverine

This map is for general reference only. The US Fish and Wildlife Service is not responsible for the accuracy or currentness of the base data shown on this map. All wetlands related data should be used in accordance with the layer metadata found on the Wetlands Mapper web site.

> National Wetlands Inventory (NWI) This page was produced by the NWI mapper





Appendix A

Boring/Well Construction Logs

	· · · · · · · · · · · · · · · · · · ·			
83296	4	LO	G OF BORING	
PROJEC CLIENT	T: Waste Water SWEPCO	Ponds		BORING NO.: MW-8 LOCATION: Hallsville
Date: 1	10-4-83	Type:	Auger	Ground Elevation:
	Legend:			
Depth, Feet Symbol	Sample Sample		X Penetratio	n ▼ Water
			Description of	of Stratum
-10- -15- -20-	Firm tan and	d grey a	silty sandy layey silty	clay w/iron ore
-25-	Very stiff g	rey silt	y clay lens	3es
_30X	43-7=5눌" 50 1	3/5½"		
35 X	Very dense ta	n and g	rey silty s	and 50 B/5½" .
-40	Bottom of bor			
-50				

. .

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1

APEX PROJECT N	O.: 310-089	BORING NUMBER:	MONITOR WELL MONITOR WELL NUMBER:	AD-16	_
FACILITY NAME:	AEP- Pirkey Power Plant		FACILITY ID NO.: N/A		_
FACILITY ADDRE	SS: Hallsville, Texas				_
DRILLING COMPA	NY/METHOD/RIG: Apex	Geoscience Inc. / Hollow-st	em Augers/ CME-55 Track Rig		_
DRILLER: Ed W	ilson, Apex Geoscience Inc.	co	OMPLETION DATE: _12/30/2010		
PREPARED BY: Jef	f Sammons		LOGGED BY: Matt Lyon/Jeff Sammons		_
LATTITUDE: N 32		a: WGS-84	WELL LOCATION: North of Mine Haul Road		
LONGITUDE: W94°					
DEPTH (FEET) PID (PPM) SAMPLE	WELL LOG AND COMPLETION DETAILS	USCS CODE	SOIL DESCRIPTION AND COMMENTS	Odor	Moisture
1 2	0-2	SM Silty sand, ver	y fine grained, light brown	None	Dry
3 4 5 6 7 8	2-8		Howish brown, reddish brown de concretions at 2.5'	None	Dry
9	8-10	CL Clay, red, light	yellowish brown, gray, fat, hard, some very fine	None	Dry
11	10-11		I, light gray, yellowish brown, stiff to hard	None	Dry
12	11-14.		h brown, some sand, reddish brown, light gray,	None	Dry
14	14.25-	-clayey sand se	am at 14-14.25', yellowish brown, light gray		Moist
15 16 17 18	14.25-	18 CL Sandy clay, red in clay	l, light gray, gray, very thin sand lenses interbedded	None	Moist to V. Moist
19 20 21 22 23 24 25	▼ 18-29.	at 21' -sandy 22', 22.	tray, light gray, yellowish brown, hard, gray wn iron oxide cemented sand laminations at 19.75' gravelly sand lense, very fine gypsum crystals 5', 24' h brown at 24-24.5'	None	Moist V. Moist
26 27 28 29 30 31 32		- dark gray, ver hard, dry, at 25	y fine gypsum crystals, trace sand,		Dry
33 34	32-35	SC Clayey sand, gr crystals, dense	eenish gray, light gray, some very fine gypsum	None	Moist
35		 			
37 38 39 40		Boring Termin	ated at 35'		
	Cement	Bentomte	Filter Sand V Water Level		
Apex geoscience inc.	Filter Sand (Size Grout (Type	Depth: 35 feet /Interval): 13-35' /Interval): Grout from 0-2 Completion	Riser Interval: Screen Interval: Screen Interval: Water level: Above Ground	+3 (ags)-15' 15-35' 23.37' 3'	

APEX PROJ	ECT NO.:	110-089		BORIN	BORING MONITOR WELL G NUMBER: MONITOR WELL NUMBER:	AD-23	_
FACILITY!	NAME:	AEP- Pirkey Power	Plant		FACILITY ID NO.: N/A		_
FACILITY A	ADDRESS:	Hallsville, Texas					_
DRILLING	COMPAN	/METHOD/RIG:	Apex Geo	science In	c. / Hollow-stem Augers/ CME-55 Track Rig		-
DRILLER:	Ed Wils	on, Apex Geoscience I	ne.		COMPLETION DATE: 12/15/2010		_
PREPARED	BY: David	Bedford			LOGGED BY: David Bedford		-
LATTITUDE LONGITUDE			Datum: W	GS-84	WELL LOCATION:		_
		413		1	•		
(FEET) PID (PPM)	SAMPLE	WELL LOG COMPLETION D		USCS CODE	SOIL DESCRIPTION AND COMMENTS	Odor	Moisture
1 2 3 4 5 6 7 8			0-9	SC	Clayey sand, brown, with yellowish brown and orangish brown laminations, very fine grained, very silty, few light gray clay streaks	None	Moist
9 10 11 12 13 14			9-14	ML	Siltstone with light gray clay streaks, light gray with orangish brown streaks, few small iron ore pebbles	None	Moist
15 16 17 18 19 20				SM	Sand, light gray with orangish brown streaks, very silty, very fine grained, few clay laminations	None	Slightly Moist
21 22 23 24 25 26 27		V		ML	Siltstone, light gray with orangish brown streaks	None None	Very Moist V. Moist
28 29 30			27-30.5	SM	Sand, light brown mottled with orangish brown, very fine grained, very silty	None	Wet
31 32		∀	30.5-31.5		Slightly sandy clay, orangish brown mottle with orangish brown, silty, very fine grained (30-31.5')	None	Moist
33 34 35			31.5-35		Lean clay, dense, small sandy streaks, dark gray, very fine grained (31.5-35')	None	Moist
36 37 38 39 40					Boring Terminated at 35'		
		Cement			Bentonite Fifter Sand Wat	er Level	
△Apex geo inc.	science	Gr	Total De and (Size/II out (Type/II Surface Co	nterval): nterval):	Riser Interval: 13-35' Screen Interval: Grout from 0-2'; Bentonite from 2-13' Water level: Elizab Above Ground	+3 (ags)-15' 15-35' 30.83	

Note: This log is not to be used separate from this report.

APEX PROJECT NO	D.: 110-089		BORING MONITOR WELL NUMBER: MONITOR WELL NUMBER: AD-24	
FACILITY NAME:	AEP- Pirkey Power Plant		FACILITY ID NO.: N/A	
FACILITY ADDRES	S: Hallsville, Texas			
DRILLING COMPA	NY/METHOD/RIG: Apex G	eoscience Inc.	:. / Hollow-stern Augers/ CME-55 Track Rig	
DRILLER: Ed Wil	son, Apex Geoscience Inc.		COMPLETION DATE: 12/27/2010	
PREPARED BY: Jeff	Sammons		LOGGED BY: Jeff Sammons	
LATTITUDE: N 32°2 LONGITUDE; W94°2	9.940'	WGS-84	WELL LOCATION: South of LF pond dam	
(FEET) PID (PPM) SAMPLE INTERVAL	WELL LOG AND COMPLETION DETAILS	USCS CODE	SOIL DESCRIPTION AND COMMENTS Odor M	loisture
1 2	0-1.5	brov	rown, medium dense	Moist
3 4	1.5-6.5	vers	ery fine grained, dense	Dry
5	11111	mat	atter at 6', gray, dark gray, reddish brown, loose to med, dense	Moist
7 8	6.5-16	SM Silty	lity sand, very fine grained, red, loose, trace clay None Sa	iturated
9 10 11 12 13			ome gravel at 10' ard cemented sandstone with iron oxide at 1!'	
15		- SOI	some clay at 15-16', medium dense, gray, dark gray	Moist
17 18 19 20	16-20		gnite, black, loose, saturated at 16-17' None Saturated at 17-20'	turated
21 22 23 24 25 26 27 28 29 30		Bori	oring Terminated @ 20'	
	Cement	/////// Bent	entonite Filter Sand V Water Level	
🛮 Apex geoscience inc.	Filter Sand (Size/I Grout (Type/I			

APEX	PROJI	ECT NO.	: 110-	089			BORIN	BORING MONITOR WELL G NUMBER: MONITOR WELL NUMBER: AD-25	<u>. </u>
FACIL	LITY N	AME:	AEP	- Pirkey	Power	Plant		FACILITY ID NO.: N/A	_
FACIL	JTY A	DDRESS	: Halls	ville, To	xas				_
DRILI	ING C	OMPAN	Y/ME	THOD/	RIG:	Apex G	eoscience	Inc. / Hollow-stem Augers/ CME-55 Track Rig	
DRILL	ER;	Ed Wils	on, Ap	ex Geos	cience I	nc.		COMPLETION DATE: 12/14/2010	<u></u>
PREPA	ARED I	Y: David	l Bedfo	rd				LOGGED BY: David Bedford	_
LATTI	TUDE:	N 32°27 : W94°29	"17.2" "59 1"			Datum;	WGS-84	WELL LOCATION: S. of Diesel ASTs	_
<u> </u>								_	
DEPTH (FEET)	PID (PPM)	SAMPLE		WELL I			USCS CODE	SOIL DESCRIPTION AND COMMENTS Odor	Moisture
				F					
1						0-1.5	SC	Clayey sand, brown, silty, very fine grained, moist None	Moist
2 3 4 5 6						1.5-7	СН	Fat sandy clay, orangish brown, very fine grained, moist None	Moist
8 9 10 11 12						7-30	sc	Clayey sand, orangish brown mottled with dark gray, very fine grained, few light gray clay inclusions	Moist
14 15 16 17 18 19 20 21 22 23 24 25 26								Wet @ 14' 15-20' - few pieces of dark gray cystalline rock	Wet
27 28 29 30									
31 32 33 34 35								Boring Terminated at 30'	
				Cement				Bentonite Fifter Sand V Water Level	
	Aper cience			Filt		d (Size/I	Depth: nterval): nterval);		-
9							Completio		- Į

				_	_						
APEX	PROJI	ECT NO.	: 110	-089				BORIN	BORING MONITOR WELL IG NUMBER: MONITOR WELL NUMBER:	AD-26	_
FACII	LITY N	AME:	AEI	P- Pir	rkey I	Power	Plant		FACILITY ID NO.: N/A		_
FACII	LITY A	DDRESS	: Hall	sville	e, Tex	Kas					_
DRILI	LING C	OMPAN	Y/ME	THO	DD/R	IG;	Apex C	eoscience	Inc. / Hollow-stem Augers/ CME-55 Track Rig		_
DRILI	LER:	Ed Wils	on, Ap	ex C	eosc	ience	Inc.		COMPLETION DATE: 12/14/2010		_
PREPA	ARED E	Y: David	Bedfi	ord					LOGGED BY: David Bedford		_
		N 32°27					Datum:	WGS-84	WELL LOCATION: By silt fence and plastic lined tren	ch	_
LONG		: W94°29	754,8								
DEPTH (FEET)	PID (PPM)	SAMPLE	CC			OG A	ND ETAILS	USCS	SOIL DESCRIPTION AND COMMENTS	Odor	Moisture
					F						
1				_8			0-1	SC	Slightly clayey sand, light brown, very fine grained, silty	None	Moist
2						///	1-3	CL	Sandy clay, lean, very fine grained, reddish brown	None	Moist
4 5							3-5	CL	Lean, slightly sandy clay with clayey sand streaks, orangish brown, small coal pieces, very fine grained	None	Moist
6							5-7	SC	Clayey sand, orangish brown, very fine grained, brown clay	None	Moist
8			 				7-14	CL	Inclusions Lean clay, orangish brown mottled with brown/light gray clayey	None	Moist
9									sand streaks, very fine grained, few coal pieces		
11			1	肼		圃					
12					! -	##	·				
14			1_							_	
15 16						▦	14-28	SC	Clayey sand, dark brown with orangish brown streaks, very fine grained, coal pieces	None	Slightly Wet
17			1						Total Figures, som proces		1101
18 19					I —	-					
20			▽	雔						- 1	Wet
21	ĺ			肼	I				20-22' - Large clay inclusions, brown	ļ	V
23											Very Moist
24					!					- 1	
26	1			H				•	Wet from 25-26'		
27				翢	.						
29 30							28-30	CL	Lean clay, light gray with orangish brown streaks, few small	None	V. Moist
31							30-32	SC	sandy streaks, very fine grained Clayey sand, light black, very fine grained, small pieces mica	None	Wet
32				鬪		1111	32-40	CL	Wet from 30-30.5' Lean clay, orangish brown, small clayey sand streaks, very	None	Moist Moist
34 35	- 1						- 1		fine grained, brown streaks		
36				Ш						ľ	Slightly Wet
37	- 1						ŀ				
38	1				Н		ľ				
40											
		70			_				Boring Terminated at 40'		 [
		- 1		Cer	nent		ž		Bentonite Filter Sand V Water Leve	1	-
D	SApex				TO LA	C-		Depth:		3 (ags)-10	
	ence:							nterval): nterval):	8-40' Screen Interval: Grout from 0-2'; Bentonite from 2-8' Water level:	19.45	- 1
9-2-6								ompletto		31	1

Note: This log is not to be used separate from this report.

APEX	PROJE	CT NO.:	110-	089				D BORIN	BORING MONITOR WELL NG NUMBER: MONITOR WELL NUMBER: AD-27	_
FACII	LITY N	AME:	AEP	- Pirke	y Po	ower	Plant		FACILITY ID NO.: N/A	_
FACII	LITY A	DDRESS	: <u>Halis</u>	sville, 1	Геха	as		_		_
DRILI	LING C	OMPAN	Y/ME1	LHOD	/RI	G:	Apex C	ieoscience	e Inc. / Hollow-stem Augers/ CME-55 Track Rig	_
DRILL	LER:	Ed Wils	on, Ap	ex Geo	scie	ence	Inc.		COMPLETION DATE: 12/15/2010	
PREP	ARED E	Y: David	Bedfo	rrd					LOGGED BY: David Bedford	_
		N 32°27					Datum:	WGS-84	WELL LOCATION: By corner lined ditch	
LONG	ITUDE	W94°29	47.3"						-	•
DEPTH (FEET)	PID (PPM)	SAMPLE	CO	WELI MPLE		-	AND ETAILS	USCS	I SOIL DESCRIPTION AND COMMENTS I Odor	Moisture
1 2							0-2	SC	Clayey sand, orangish brown with dark gray laminations, None very fine grained	Slightly Moist
3 4 5 6 7							2-15,5	CL	Lean clay, dense, few thin sandy streaks, reddish orange, very fine grained, mottled with light brownish gray	Slightly Moist
8 9 10 11 12 13									Orange to brown with orangish brown streaks, at 10' becomes brittle	
15 16 17 18 19 20 21 22 23							15.5-23	sc	Clayey sand, greenish brown with orangish brown streaks, few None thin tan clay streaks, very fine grained	Moist
24							23-24	SM	Sand, orangish brown, silty, very fine grained None	Wet
25 26 27			∇.				24-27	CH	Fat clay, brown with orangish brown streaks, many sandy Streaks, very fine grained None	Very Moist
28 29 30							27-30	SM	Sand, greenish gray with orangish brown streaks, very fine to None fine grained, wet	Wet
31 32 33 34 35				幽			30-37.5	SC	Clayey sand with clay streaks, light greenish black, very fine None grained	Slightly Wet
36 37							i		Wet red brittle shale from 35-35,2	
38 39 40									Boring Terminated at 37.5'	
				Ceme	nt				Bentonite Filter Sand V Water Level	
	Ape: clence			F		Grou	d (Size/) t (Type/)	-	15.5-37.5' Screen Interval: 17.5-37.5' Grout from 0-2', Bentonite from 2-15.5' Water level: 26.73'	

Note: This log is not to be used separate from this report.



Monitor Well

Monitor Well No.: AD-34

PROJECT INFORMATION

PROJECT:

Pirkey Power Plant

PROJECT NO .: I-04-1021

LOGGED BY:

Jeffrey D. Sammons, P.G. SUPERVISING PG: Jeffrey D. Sammons, P.G.

COMPLETION: DEVELOPMENT: 12/11/2015 12/16/2015

SITE LOCATION: 2400 FM 3251, Hallsville, Texas

WELL OWNER: AEP

DRILLING INFORMATION

Buford Collier

DRILLER: DRILLER'S LICENSE NO.: 50089

RIG TYPE: Geoprobe 3230DT

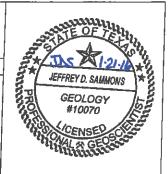
METHOD OF DRILLING: Hollow Stem Auger

SAMPLING METHODS: Split Core SURFACE ELEVATION:

307.61 (Top of Casing) HOLE DIAMETER:

8.26*

LATITUDE 32 27' 10.13" LONGITUDE 94 29' 57.93"



Page 1 of 1

				27 IU.	13 L				79.9 1			
Water Level Upon Installation	Water Lev	el at Tir	ne of D	rilling		,	Geo	techn	ical L	ab Sa	mple Ţ	BPG No. 50027
DESCRIPTION	nscs	SYMBOLS	DEPTH	WATER	SAMPLE	% MOISTURE	% FINES	7	4	٦		WELL. CONSTRUCTION
CLAYEY SAND: very fine to fine sand, some silt, reddish brownish, light reddish brown, light gray, moist FAT CLAY: trace sand and silt, some iron oxide concretions, dark reddish brown, reddish brown, and and light gray, moist - some silt and very fine to fine sand at 5', light gray, light reddish brown, and light yellowish brown, moist to very moist SANDY LEAN CLAY: some very fine to fine sand, dark gray, moist - reddish brown, dark reddish brown, dark gray, light gray at 10' to 12.5' SILTY SAND: very fine to fine sand, some clay, gray and dark gray, saturated - increasing clay content with depth	SC CH CL		-4 -3 -2 -1 -1 -2 -3 -4 -5 -6 -7 -8 -10 -11 -12 -13 -15 -16 -17 -18 -19 -20	¥	23	6	9 2	5 N		40		Locking Well Casing Cover Locking Well Cap Protective Well Casing Concrete Pad Ground Surface Cement Bentonite 2" Sch. 40 PVC Riser 20/40 Silica Sand 0.010" Slotted Sch. 40 PVC Well Screen
CLAYEY SAND: fine to very fine sand, dark gray, moist to very moist	SC		- 21 - 22 - 23		23	90	5:	5 2	7 2	8		PVC Bottom Cap
		1	24 25									

NOTES: This log should not be used separately from the original report. Not all USCS descriptors were laboratory verified.



Monitor Well

Monitor Well No.: AD-35

PROJECT INFORMATION

PROJECT: PROJECT NO .:

Pirkey Power Plant

1-04-1021

LOGGED BY:

Jeffrey D. Sammons, P.G. SUPERVISING PG: Jeffrey D. Sammons, P.G.

COMPLETION:

DEVELOPMENT:

12/11/2015 12/16/2015

SITE LOCATION: 2400 FM 3251, Hallsville, Texas WELL OWNER:

DRILLING INFORMATION

DRILLER: DRILLER'S LICENSE NO.: 50089

RIG TYPE:

SAMPLING METHODS: SURFACE ELEVATION:

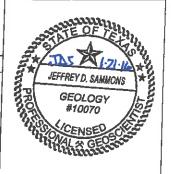
HOLE DIAMETER:

Buford Collier

Geoprobe 3230DT METHOD OF DRILLING: Hollow Stem Auger

> **Split Core** 318.95 (Top of Casing)

8.25" LATITUDE 32 27' 9.64" LONGITUDE 94 29' 42.74"



₩ Water Level Upon Installation ✓	Water Lev	el at Tin	ne of D	Prilling		1	Ge	otechi	nical (.ab Sa	mple T	BPG No. 50027
DESCRIPTION	USCS	SOIL	DEPTH	WATER	SAMPLE	% MOISTURE	% FINES	7	₫	۵		WELL CONSTRUCTION
CLAYEY SAND: very fine to fine sand, some iron ore gravel, reddish brownish, dark reddish brown, yellowish brown, gray, moist SILTY SAND: very fine to fine sand, trace clay, trace iron ore gravel, light reddish brown, moist, increasing moisture content with depth	SC		-4 -3 -2 -1 -1 -2 -3 -4 -4 -5 -8 -8	*		13	46	32	15	17		Locking Well Casin Cover Locking Well Cap Protective Well Casing Concrete Pad Ground Surface Cement 2" Sch. 40 PVC Riser Bentonite
CLAYEY SAND: very fine to fine sand, trace iron ore gravel, light reddish brown, very moist thin seams of saturated very fine sand with trace of slay at 12.25' to 12.5' light reddish brown and light gray, moist to very noist at 12.5' to 15' LEAN CLAY: intebedded clays and silts with aminations of very fine sand, light gray, gray and light reddish brown, moist to very moist thin lenses of very moist very fine sand and artially cemented very fine sand at 17.5' and 18', addish brown	SC		-10 3 -11 -12 -13 -14 -15 -18 -17	VZ	15					13		0.010" Slotted Sch. 40 PVC Well Screen

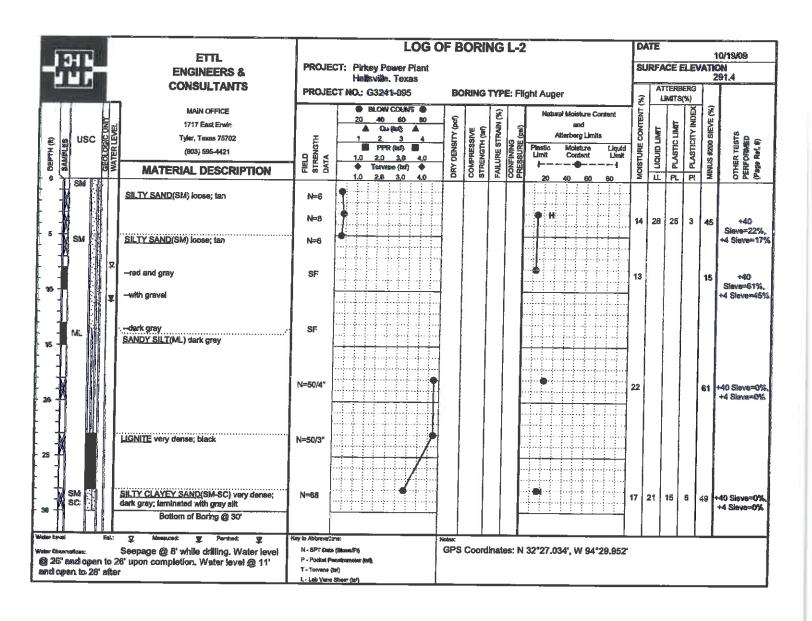
	85216	54		LOG	OF BORING	
P	ROJEC	CT:	Monitoring W Southwestern	Well Instill	ation Power Company	BORING NO.: M ←1 LOCATION: Hallsville, TX S 12+32.79; W 35+34.55
	ate:		1/29/86	Type:	Rotary	Ground Elevation: 337.67
			Legend:			
Depth, Feet	Symbol	Sample	Sample Sample		X Penetration	▼ Water
" "	"	17			Description of	Stratum
				· · · · · · · · · · · · · · · · · · ·		
5			Brown and t	an sandy cl	lay	
-10-			Brown and ta	an sandy cl	.ay w/iron ore	
15			Brown and ta	nn sandy cl	ay w/iron ore	
_ 20			Gray silty s	sand		
- 25_			Gray silty s	and		
-30-	47,		Gray silty s	and.		
-35-			Bottom of Bo Water encound	ring at 30 tered at 10	feet.) feet.	
						and the second s
40_						
						1
45_						
50_						-
	بباب					

852164			G OF BORING		
PROJECT: CLIENT:		Well Insta Electric	llation Power Company	BORING NO.: LOCATION: Ha S 38+86-22;	M.4-2 llsville TX W 45+76.41
Date:	1/29/86	Type:	Rotary	Ground Elevation:	302.19
	Legend:				
Depth, Feet Symbol	Sample Sample		X Penetration	▼ Wa	ater
			Description of	Stratum	
11:11:					
-5-	Brown silty	sand w/ir	on ore		
-1010-0	Brown silty	and relieve			
	DIOWII SIICY	Salid W/II	Oil Ore		
_15	Brown and g	ray silty	sand		
20	Gray silty :	sand			
_ 25					
35.8	Gray silty s	sand			
	D-11				
-30-	Bottom of Bo Water encour	ring at 2 itered at 1	/ reet. Ll feet.		
-35					
-35-					ĺ
- 40					
-45-					}
-50-					

		100	OF BORING		
852164 PROJECT: CLIENT:	Sludge Disposal Southwestern Ele	Area	5	BORING NO LOCATION:	D.: M. 2A Hallsville, TX
Date:	6/20/85	Type:	Auger	Ground Elevation	
Depth, Feet Symbol		nt Site	Coordinates: X Penetration	T.T. o. do	S 27+55.45 W 36+47.44 Water
DIL S) 		Description of	Stratum	
	Red-brown cla	yey sand	1	E	
10 0.08	Gravel		**		
	Gray clayey s	ilty sar	ıd		
20_	Gray silty sa	·			
-25-	Bottom of Bor Water encount Bottom of Cas Screen length	ered at ing at 2	12 feet. 2 feet.		
-30- -35-	·				
- 40-					
- 45					
50					

Company of the last of the las	_		LOG 0	FΒ	ORII	NG	L-1		DA	TE		-	10/19/09		
	EITL ENGINEERS &	PROJE	CT: Pirkey Power Plant						SU	RFA	CE E	LEV/	ATION 296.0		
الملك	CONSID TANTS		Hallsville, Texas PROJECT NO.: G3241-095 BORING TYPE: Flight Auger								ATTERBERG				
	MAIN OFFICE	T KOOL	● BLOW COUNT ●		I				8	L	MITS		2		
	1717 East Envis		20 40 60 60 A Qu (tsf) A	8	m e	FALURE STRAIN (%)	Î	Natural Moisture Content and	MOISTURE CONTENT (%)	_	Ę	PLASTICITY INDE	MINUS #200 SIEVE (%) OTHER TESTS PERFORMED (Page Ref. #)		
E SU USC SE	Tyler, Teores 75702	E	1 2 3 4	ORY DENSITY (pcf)	COMPRESSIVE STRENGTH (tsf)	STR	SH	Atterberg Limits Plastic Moisture Liquid	20 1	LIQUID LIMIT	PLASTIC LIMIT	틾	MINUS #200 SIE OTHER TESTS PERFORMED (Page Ref. #)		
SAMPLES SAMPLES CEOLOGIC WATER LEV	(903) 595-4424	FIELD STRENGTH DATA	1.0 2.0 3.0 4.0 Torvans (tsf)	3.08	MPR	13	CONFINING PRESSURE (Plastic Moleture Liquid Limit Content Limit 11	NSTUR	2	PLAS	PLAS	HER T		
0	MATERIAL DESCRIPTION	1	1.0 2.0 3.0 4.0	ő	SP	Æ	8 %	20 40 60 80	M	Щ	PL	PI	2 5 H C		
CL CL	SANDY LEAN CLAY(CL) stiff; orangish gray	N=14	7					1	15	37	19	18	74 +40 Sleve=7% +4 Sieve=2%		
# 1	-reddish brown	N=11							18	39	20	19	74 +40		
5]											Sieve=11%, +4 Sieve=3%		
													1		
SC SM	SHITY CLAYEY SAND(SC-Sk/) reddish brown; with gravel	P=4.5+						•	7	20	15	5	32 +40 Slovens4 N		
10 🗍		Į								ĺ			Sieve=61%, +4 Sieve=33%		
	-reddish tan; with iron oxide comented sandstone	8F									-				
15									Ì		-				
1 1	***************************************						ŀ								
SM	SILTY SAND(SM) very dense; dark gray; laminated; seturated	N=50/3"	7						21			3	+40 Sieve≈0% +4 Sieve≈0%		
°			/				ľ				1				
# M	SANDY SILT(ML) very dense; dark gray; with				ı										
5 1 ML	ignite @ 24'	N=73	\				ŀ					1			
~{			\\\						- 1						
1	LIGNITE very dense; black	AL-EDM EN													
. ₩	POSITIE Agià desines' twenty	N=50/0.5*								ı					
4							_ [- [

1941	ETTL ENGINEERS &	PROJEC	LOG O	FB	ORIN	G L	1	1		RFA	ACE I	ELEV	/ATIC	0/19/08 DN
F.T.	CONSULTANTS	PROJEC	Hallsville. Texas CT NO.: G3241-095	ight Auger	(%)		TTERE			96.0				
SAMPLES CO	MAIN OFFICE 1717 East Envin Tyler, Texas 75702 (903) 595-4421 MATERIAL DESCRIPTION	FIELD STRENGTH DATA	BLOW COUNT 20 40 60 36 A Qu (tan) A 1 2 3 4 PPR (tan) 1.0 2.0 3.0 4.0 1.0 2.0 3.0 4.0	DRY DENSITY (pcr)	COMPRESSIVE STRENGTH (1st)	FAILURE STRAIN (%)	CONFINING PRESSURE (ps)	Natural Moisture Content and Atterberg Limits Plestic Moisture Limit Content Limit	CONTENT	F LIQUID CIMIT		PLASTICITY INDEX	MINUS #200 SIEVE (%)	OTHER TESTS PERFORMED (Pego Rof.#)
35 - CL	LEAN CLAY(CL) hard; light gray; with vertical black silty seam	N=50/3*	(M AN QU TN					20 40 60 80	15					+40 Sieve=0%, +4 Sieve=0%
CL ML	SANDY SILT CLAY(CL-ML) hard; gray	P=2.25 P=4.5+							16	22	16	6	51	+40 Sieve=0%, +4 Sieve=0%
CL CL	LEAN CLAY(CL) hard; dark brown iaminated	P#4.5+		i										
500	Bottom of Boring @ 50'	P=4.5+					-							
We're Cheervalous: @ 18" and open u	Seepage @ 17' while drilling, Water level	Key to Abbrevesion N - SPT Date P - Postet Per T - Torvere (to	n (Blows/Ft) enoirometer (taf)	Notes:			Τ			_				

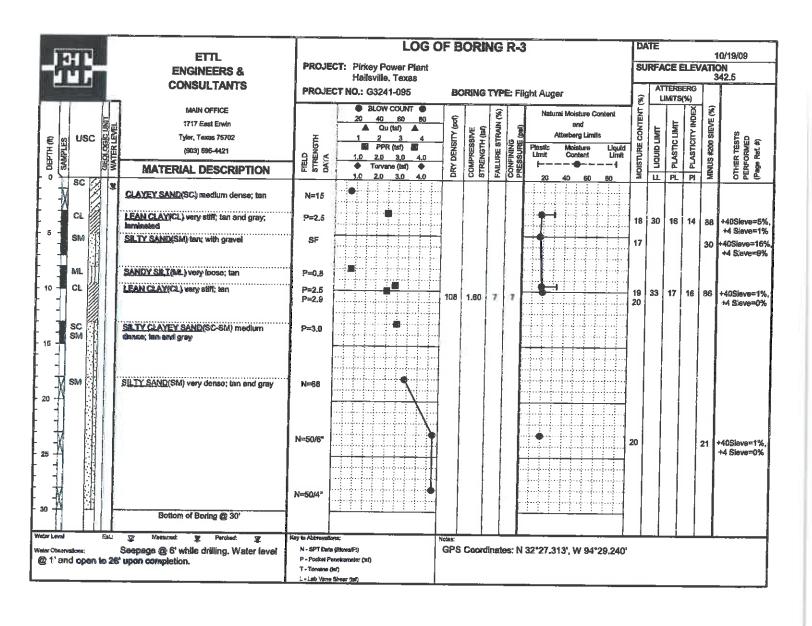


in a	EITL		LOG	OF E	ORI	NG	R-1				D/	NTE.	•		10/19/09
	ENGINEERS &	PROJE	ECT: Pirkey Power Plant Hellsville, Texas								SU	IRFA	CE E	LEV/	ATION 356.3
وناسان	CONSULTANTS	PROJE	ECT NO.: G3241-095	B	ORING	TYP	Æ: Fli	ight Aug	ger		<u>8</u>		TERBI		T
SAMPLES SAMPLES S S S S S S S S S S S S S S S S S S	MAIN CFFICE 1717 East Erwin Tyler, Texas 75702	HLE	BLOW COLINT 20 40 60 80 Cu (ts/) 1 1 2 3 4	DRY DENSITY (pcf)	SSIVE TH (bst)	FAILURE STRAIN (%)	AG RE (ps)	4	tural Moisture and Atterberg Lin Moisture	nits	CONTENT	LIQUID LIMIT	PLASTICLIMIT		MINUS #200 SIEVE (%) OTHER TESTS PERFORMED (Page Rof. #)
ال في ا	(903) 595-4421 MATERIAL DESCRIPTION	FIELD STRENGTH DATA	1.0 2.0 3.0 4.0 Torvane (tsf) 1.0 2.0 3.0 4.0	DRY DEN	COMPRESSIVE STRENGTH (br)	FAILURE	CONFINING PRESSURE (DB)	Limit 	Content 40 60	Llimit 	MOISTUR	F LIQUIC	PLAST	PLAST	MINUS #200 SIE OTHER TESTS PERFORMED (Page Rof. #)
CL SC	SANDY LEAN CLAY(CL) medium stiff; red and brown; with grave!	N=9			\Box				10 0				_		
5 Sc	-stiff; clay centent increasing CLAYEY SAND(SC) medium dense; reddish	N=13									17	47	19	28	52 +40Sieve=4% +4 Sieve=1%
	CLAYEY SAND(SC) medium dense; reddish brown; with ferric seams	P=2.75													
10	1	P=3.0									13	33	16	17	22 +40Sieve=36% +4 Sieve=30%
**	—with clay nodules	P=2.5 P=1.75 SF		106	1.10	4	9	ł			20				
10. 10. 10. 10. 10. 10. 10. 10. 10. 10.		P≍3.0							1		17	34	15	19 3	+40Sieve=8%, +4 Sieve=1%
CL S	SANDY LEAN CLAY(CL) very stiff; orange and tan; saturated	N=25 P=3.5	•						-,		18	42	21	21 5	7 +40Sieve=9%, +4 Sieve=6%
SC SM	CLAYEY SILTY SAND(SC-SM) orange and gray	SF													
oter Level Est	st.: 👽 Mosetred: 💘 Perchad: 💇	Key to Abbrovet	Money:	Notes:											

-		-1		EITL		LOG O	DATE 10/19/09									
۲Ĥ	Н	k		ENGINEERS &	PROJE	PROJECT: Pirkey Power Plant Hallsville, Texas										ON 356.3
		-1	N	CONSULTANTS	PROJE	PROJECT NO.: G3241-095			TY?	E: Fli	light Auger	<u>%</u>		ERBERG VITS(%)		
H (R)	SAMPLES	USC ON		MAIN OFFICE 1717 East Erwin Tyler, Texas 75702 (903) 595-4421	FIELD STRENGTH DATA	BLOW COUNT 20 40 60 60 Cu (ms) 4 1 2 3 4 PPR (ss) 1	DRY DENSITY (pcf)	COMPRESSIVE STRENGTH (tel)	FAILURE STRAIN (%)	VING URE (ps)	Natural Moisture Content and Atterberg Limits Plestic Moisture Liquid Limit Content Limit	MOISTURE CONTENT (LIQUID LIMIT	PLASTIC LIMIT	MINUS #200 SIEVE (%)	OTHER TESTS PERFORMED (Page Ref. #)
SAME	OVINI		WATED I EV	MATERIAL DESCRIPTION	FIELD	1.0 2.0 3.0 4.0	DRY D	COMPI	FAILUR	CONFINING PRESSURE (1 20 40 60 80	MOIST	9	PL PI	MINIUS	OTHER PERFO (Page F
35	SI	A COLUMN		SILTY SAND(SM) very dense; gray; with gravel	N=50/5,5°	7					•	22			16	
40	V V			~crange and gray	N=60											
45				brown	N=36											
50				-brown and gray	N≈50/5.75°											
				Bottom of Boring @ 50'				A PARTIE AND THE PART								
fator Leve	e d		Est.	도 Messumd: 및 Perched: 및 Seepage @ 25' While drilling.	Kay to Abbreves		Notes:									

	-				LOG ()FB	ORI	NG	R-2	?		D/	AFE			-	10/19/09		
",i	4		ETTL ENGINEERS &	PROJE	CT: Pirkey Power Plant Hallsville, Texas							SI	SURFACE ELEVATION 355.1						
Į.i.			CONSULTANTS	PROJE	PROJECT NO.: G3241-095 BORING TYPE; Flight Auger										ERG (%)	Τ	55.1		
DEPTH (ft) SAMPLES	usc	GEOLOGICUNIT WATER LEVEL	MAIN OFFICE 1717 East Erwin Tyler, Texas 75702 (903) 595-4421 MATERIAL DESCRIPTION	FIELD STRENGTH DATA	BLOW COUNT 20 40 80 80 Cu (tst) 1 2 3 4 PPR (tst) 1.0 2.0 3.0 4.0 Torvane (tst)	DRY DENSITY (pcf)	COMPRESSIVE STRENGTH (br)	FAILURE STRAIN (%)	CONFINING PRESSURE (psi)		and Moisture Content and Atterberg Limits Moisture Limit Content Limit	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	MINUS #200 SIEVE (%)	OTHER TESTS PERFORMED (Page Ref. #)		
	sc /		CLAYEY SAND(SC) medium dense; reddish	P=4.5+	1,0 2.0 3.0 4,0	-	0 6	-	0 2	20	40 60 80		_	PL	PI				
1			tan; with gravel	P=4.5+	•							13	38	17	21	44	+40Sieve=6%, +4 Sieve=2%		
5 -			-red and orangish gray; with day lenses	P=4.5	•					1		14	38	17	21	41	+40Sieve=9%, +4 Sieve=3%		
1			-gravelly and ferric seams	P=2.0						å.		18	36	18	18	44	+40Sieve=12%, +4 Sieve=5%		
10				₽=3.5													+4 SI8V6=5%		
15			-orange and red	P≈3.0 P≈4.0															
20																	1		
25			-red and tan	P=4.5+								17	43	18	25	42	+40Sieve=6%, +4 Sieve=0%		
30			-red and orange	P≖4.0															
Pater Creary	5/-2	Est.:	्र Messured: प्र Pecked: प्र Seepage @ 38' while drilling.	N - SPT Date P - Pocket P T - Torverse L - Lab Varia	n (BlovenFi) 'enselversoler (luf) (luf)	Notes:				<u> </u>									

Description of the last of the	EITL		DATE 10/19/09												
134 k	ENGINEERS &	PROJECT: Pirkey Power Plant Hallsville, Texas								SURFACE ELEVATION 355.1					
adit-did	CONSULTANTS	PROJECT NO.: G3241-095		ВС	DRING	TYP	E: Fli	ight Auger			ITERBERG LIMITS(%)				
SAMPLES SAMPLES C C C C C C C C C C C C C	MAIN OFFICE 1717 East Envin Tyles, Taken 75702 (903) 598-4421	FIELD STRENGTH DATA	BLCW COUNT 20 46 60 80 Qu (lsf) 1 2 3 4 PPR (lsf) 1.0 2.9 3.0 4.0	DRY DENSITY (pcl)	COMPRESSIVE STRENGTH (tst)	FAILURE STRAIN (%)	CONFINING PRESSURE (pol)	An	Moisture Content and enterg Limits Moisture Liquid Content Limit	CONTENT	ISTURE CONTENT	PLASTIC LIMIT	PLASTICITY INDEX	MINUS #200 SIEVE (%) OTHER TESTS PERFORMED (Page Ref. #)	
- \ 	MATERIAL DESCRIPTION	E 22	1.6 2.0 3.0 4.0	E E	8 8	¥	9.5	20 4	40 80 80	MO	ш	PL	Pi	PER C	
35 - V	SANDY LEAM CLAY(CL) soft; ten and gray	P=4.5 P=0.2								22	34	15	19	403ieve=7%, +4 Sieve=3%	
35 - CL	stiff; gray and tan	P=1.3													
50 SM	Sil TY SAND(SM) gray Bottom of Boring @ 50'	SF													
Value Lavel Est.	∑ bileanswed: ∰ Perchad: ∰	Key to Abbraves		Notes:											



Appendix B

Photographic Log



PHOTOGRAPHIC LOG

Project Name:

AEP - Pirkey Power Plant

Location:

Hallsville, Harrison County, Texas

Project No.

OH015976.0001

Photo No.

Date: 8/19/2015

Direction Photo Taken:

South

Description:

P8190454 Upland drainage area along southeastern side of Landfill.



ARCADIS

PHOTOGRAPHIC LOG

Project Name:

AEP - Pirkey Power Plant

Location:

Hallsville, Harrison County, Texas

Project No.

OH015976.0001

Photo No.

Date: 8/19/2015

Direction Photo Taken:

South

Description:

P8190467 Lining for new landfill expansion cell





PHOTOGRAPHIC LOG

Project Name:

AEP - Pirkey Power Plant

Location:

Hallsville, Harrison County, Texas

Project No.

OH015976.0001

Photo No.

Date: 8/19/2015

Direction Photo Taken:

East Northeast

Description:

P8190470 Lining for new landfill expansion cell



ARCADIS

PHOTOGRAPHIC LOG

Project Name:

AEP - Pirkey Power Plant

Location:

Hallsville, Harrison County, Texas

Project No.

OH015976.0001

Photo No.

Date: 8/19/2015

Direction Photo Taken:

East Northeast

Description:

P8190475 Upland ditch on east side of landfill.



Appendix C

Soil Boring Logs and Piezometers - 2018 Landfill Lateral Expansion Area



6		Auck	dan	d Consulting LLC				LO	G O	FI	301	RIN	IG I	31
Pro	ject I		on:	key - 2019 Landfill Expansion Hallsville, Texas : C&S Lease					oject			09		6-011 '2016
GPS	Coo	rdinat	es: N	i32° 27' 12.0" W94° 29' 48.6"										
Suri	face I	Elevati	ion;	N/A		er (ts) (tsf	8	9				٦	
Dril	ling l	Metho	d: H	SA	E.	mete	engt	ieve	nt (9				it (pc	
Groundwater Elevation (ft)	Depth (feet)	Sample Type	Graphic Log	Material Description	N-Value (Blows/ft)	Pocket Penetrometer (tsf)	Unconfined Strength (tsf)	Passing #200 Sleve (%)	Moisture Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	Unit Dry Weight (pcf)	
<u> </u>	0	_	270			_				_				
-		\boxtimes		Medium Stiff, red, brown and gray, Sandy Lean Clay (CL), mottled, few gravel	8		İ	52	16	39	13	26		
	-5	\times	, , , , ,	Gray, Bottom Ash	13	N/A			15				64	:
Y	- 10	\boxtimes			6									
	- 15	×		Very Loose, brown, gray and red, Silty Clayey Sand (SC-SM), mottled, with organics (roots)	3			49	24	22	16	6	1	
	- 20	\boxtimes		- red, tan and gray at 18 ft	1									
	- 25		444444	Very Loose, light gray, red and tan, Silty Sand (SM), mottled	1	N/A		36	19	NP	NP	NP		
-	- 30	×	######################################	- loose, few clay below 28 ft	10			31	19					
- - -	35	\boxtimes		Very Stiff, dark gray and gray, Sandy Lean Clay (CL)	24			55	21	29	13	16		
	40	\times		Black, Lignite	50/2									
	45	\times		Hard, dark gray and gray, Lean Clay (CL), laminated, few lignitic material	61	4.5		97	20	38	20	18	105	
-	50			Boring terminated at 47 feet.		ŀ								

Additional Information/Comments:

Logger: D. Diduch

Notes/Comments: Seepage encountered at 8 ft during drilling. Water level at 8 feet upon completion.



6		Auc	klan	d Consulting LLC				LO	G C	F	301	RIN	[G]	B2
P	roject	Locat	ion:	rkey - 2019 Landfill Expansion Hallsville, Texas					ojec			-		6-011
	rilling				1		1	Di	ill D	ate(s	;): 	1 05	7/06	/2016
G	PS Coo	rdina	tes: l	N32° 27' 12.3" W94° 29' 45.3"										
Su	irface :	Eleva	tion:	N/A		3	13	18						
Dı	rilling	Metho	od: H	ISA	æ	nete	lg l	yve (1 %				(pc	
5.2	,	0			- MS/	tron	Stre	0 Sie	nten			lex lex	ig.	
Groundwater Elevation (ft)	Depth (feet)	Sample Type	Graphic Log	Material Description	N-Value (Blows/ft)	Pocket Penetrometer (tsf)	Unconfined Strength (tsf)	Passing #200 Sieve (%)	Moisture Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	Unit Dry Weight (pcf)	
	0			Very Stiff, red and brown, Sandy Lean Clay (CL), mottled, few gravel		2.5		57	18	30	12	18		
	F _	\geq	Y V	Gray, Bottom Ash	4									
	<u>-5</u>	\times			3									
T	10	\geq	W		1									
	Ė		000											
	15	×	227		2									
	- - - 20			Soft, dark gray, Sandy Lean Clay (CL)		0.5	;	62	20	34	13	21	91	
	- 25	\boxtimes		- very soft below 23 ft	1									
	- 30			Very Stiff, dark gray and gray, Sandy Silty Clay (CL-ML), with lignitic material		3.0		50	15	20	16	4	117	
	- 30	\geq		Black, Lignite	50/3	ļ								
	- - - 35	\boxtimes			50/3									
	40	\boxtimes		Hard, gray and dark gray, Lean Clay (CL), laminated, trace gypsum	44	,		95	22	38	17	21		
	-			Boring terminated at 40 feet.										
	- - 4 5													
	}													

Additional Information/Comments:

Logger: D. Diduch

Notes/Comments: Seepage encountered at 8 ft during drilling. Water level at 8 feet upon completion.

6		Auck	dan	d Consulting LLC)	LO	G O	FE	OF	NIS	G I	33
P	roject	Locati	on:	key - 2019 Landfill Expansion Hallsville, Texas : C&S Lease	1		1			No.:		09		6-011 /2016
Si	PS Coo urface rilling	Elevat	ion:		/ft)	meter (tsf)	ength (tsf)	eve (%)	nt (%)				t (pcf)	
Groundwater Flevation (#)	Depth (feet)	Sample Type	Graphic Log	Material Description	N-Value (Blows/ft)	Pocket Penetrometer (tsf)	Unconfined Strength (tsf)	Passing #200 Sieve (%)	Moisture Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	Unit Dry Weight (pcf)	
¥	-5	×		Soft, brown, tan and red, Sandy Silty Clay (CL-ML), mottled, with few organics	4	N/A N/A		53	19	20	15	5		
	10	\times		Very Stiff, brown and gray, Lean Clay with Sand (CL), mottled, laminated	24	<u> </u> 		77	19	29	17	12		
	- 15 - 15			- stiff below 13 ft		1.5		75	22	31	17	14	103	
	20	\times		Black, Lignite	50/3.75									
	[- 25 -	\boxtimes		Hard, gray, Lean Clay (CL), mottled, laminated, few lignitic material	55		l							
	- 30	×		- very stiff between 28 ft and 30 ft - hard below 30 ft	33	2.5		99	17	38	18	20	117	
	- 35 -	×		- attempted, no recovery Very Stiff, gray, Fat Clay (CH), mottled, laminated, trace gypsum	27	N/A		97	23	51	17	34		
	- 40 -	\boxtimes		Boring terminated at 40 feet.	26									
	- - 45 - -												į	
	- _ 50													

Additional information/Comments:

Logger: D. Diduch

Notes/Comments: Seepage encountered at 5 ft during drilling. Water level at 5 feet upon completion.



	A	uck	land	d Consulting LLC]	LO	G O	F B	OF	RIN	G E	34
Proje		catio	n:	key - 2019 Landfill Expansion Hallsville, Texas C&S Lease			,		-	No.:				5-011 2016
Surfa	Coordi ice Ele ing Me	vati	on:	•	/æ)	meter (tsf)	ength (tsf)	leve (%)	nt (%)				t (pef)	
Groundwater Elevation (ft)	Depth (feet)	Sample Type	Graphic Log	Material Description	N-Value (Blows/ft)	Pocket Penetrometer (tsf)	Unconfined Strength (tsf)	Passing #200 Sieve (%)	Moisture Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	Unit Dry Weight (pcf)	
	5	X		Very Loose, reddish brown, Clayey Sand (SC), mottled - medium dense between 3 ft and 10 ft - few gravel below 5 ft	2 15	3.5		27	14	23	14	9		
Y	10	×		- loose between 10 ft and 15 ft	5	2.5		49	18	28	13	15	115	
	15 20	X X		- medium dense between 15 ft and 18 ft - very dense below 18 ft	27 60	0.5		49	20	29	15	14		
	25	Z Z		Hard, gray and dark gray, Lean Clay (CL), mottled, laminated - very stiff at 28 ft	32 17	;		88	22	45	16	29		
	35			- attempted, no recovery Very Dense, gray and tan, Silty Sand (SM),	50/2	N/A								
-4 -4 -1	10	<u> </u>		with lignitic material Hard, gray, dark gray and black, Lean Clay (CL), mottled, with lignitic material Boring terminated at 40 feet.	95/11									
	15													

Additional information/Comments:

Logger: D. Diduch

Notes/Comments: Seepage encountered at 10 ft during drilling. Water level at 10 feet upon completion.



LOG OF BORING B5

2016-011

Pr	oje	ct l	Vame:	Pirkey -	2019	9 Landfill	Expansion
_			_			town.	

Project Location:	Hallsville, Texas	•	Project No.:	
Drilling Contracto	r: C&S Lease		Drill Date(s): (J

Drilling Contractor: C&S Lease			_	Dril	l Dat	te(s)	:	09/	09/2	2016
GPS Coordinates: N32° 27' 9.2" W94° 29' 42.4"		Ç.								
Surface Elevation: N/A		프	(tsf)	(%)	(%				윤	
Drilling Method: HSA	£(meter	ength	eve	nt (%	ĺ	İ		ا قِ ا	
	SW.S	ا ۾	S.	0 Si	nte	_,		dex	igh	

Groundwater Elevation (ft)	Depth (feet)	Sample Type	Graphic Log	Material Description	N-Value (Blows/	Pocket Penetrom	Unconfined Strer	Passing #200 Sie	Moisture Conten	Liquid Limit	Plastic Limit	Plasticity Index	Unit Dry Weight	
	0			Losse weddiek haven Clause Card (CO)	7	Т	Τ-				1	T		
	-	\geq		Loose, reddish brown, Clayey Sand (SC), mottled	8									
	- 5 -	\times		Very Stiff, red and brown, Sandy Lean Clay (CL), mottled, few gravel	19	4.0	6.6	51	16	34	14	20	116	
	- 10 	×		Dense, red, tan and brown, Clayey Sand (SC), mottled	34	4.0		40	20					
*	- 15 	\boxtimes		Very Stiff, gray and tan, Lean Clay (CL), mottled, laminated	17			96	21	41	16	25		
	- 20 	×		Very Stiff, gray and dark gray, Lean Clay with Sand (CL), mottled	38	3.0		55	18	29	17	12		
	- - - 25 -	\boxtimes		Black, Lignite	50/6									
	- - 30	×		Hard, gray and dark gray, Lean Clay (CL), mottled, laminated	39	4.0		99	21					
	35	\boxtimes		- very stiff at 33 ft	24									
	- 40	×		Medium Dense, gray and dark gray, Clayey Sand (SC)	27	2.5		45	27	36	16	20		
	- 45			Boring terminated at 42 feet.		ļ								
E	_50													

Additional Information/Comments:

Logger: D. Diduch

Notes/Comments: Seepage encountered at 13 ft during drilling. Water level at 13 feet upon completion.



		Aucl	dan	d Consulting LLC				LO	G O	F	3 0 1	RIN	G E	36
Pr	oject l		on:	key - 2019 Landfill Expansion Hallsville, Texas : C&S Lease			Ţ-		oject			09		5-011 2016
Sur	rface l	rdinat Elevat Metho	ion:	•	s/ft)	ometer (tsf)	rength (tsf)	Sieve (%)	ent (%)			×	ht (pcf)	
Groundwater Elevation (ft)	Depth (feet)	Sample Type	Graphic Log	Material Description	N-Value (Blows/ft)	Pocket Penetrometer (tsf)	Unconfined Strength (tsf)	Passing #200 Sieve (%)	Moisture Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	Unit Dry Weight (pcf)	
	- 0 - - -5	×		Very Stiff, red and brown, Sandy Fat Clay (CH), mottled	17	4.5		55	19	61	25	36		
	10	×		Medium Dense, light red, tan and brown, Silty Clayey Sand (SC-SM), mottled - tan, light red and gray below 8 ft, with few gravel between 8 ft and 10 ft	19	3.5		48	11					
	- 15 - 20	× ×	11,11,11,11,11,11,11,11,11,11,11,11,11,	Very Dense, tan, light red and gray, Silty Sand (SM), mottled	88/10 42	2.5		33	12	19	15	4		
	- 25 - 25	×	5555555 444643333 4449444	- dense between 18 ft and 23 ft - medium dense between 23 ft and 28 ft	28									
Y	30	X		- very dense below 28 ft Hard, gray and dark gray, Lean Clay (CL),	85/11			17	22	NP	NP	NP		
	- 35 - - - - - 40	×		mottled, laminated - little recovery, few sand between 38 ft and 40 ft	48	N/A		97	22	42	17	25	ļ	
	- 45	X		- gray, dark gray and black, with lignitic material, laminated between 43 ft and 45 ft	50/3									
	- 50 - 55	\simeq			60									
	- 60	×		Boring terminated at 60 feet.	40			98	19	42	17	25		
E	_ 65		L		<u> </u>			1	ĺ					

Additional Information/Comments:

Logger: D. Diduch

Notes/Comments: Seepage encountered at 28 ft during drilling. Water level at 28 feet upon completion.



LOG OF BORING B7

Project Name:	Pirkey -	2019	Landfill	Expansion

C&S Lease

•	•	-
Project Location:	Hallsville, Texas	

Project No.: Drill Date(s): 2016-011

09/14/2016

GPS Coordinates: N32° 27' 7	1"

W94°	29'	44.6"
		1 110

Su	rface E	levat	ion:	N/A
Dr	illing N	letho	d: H	ISA
H GD		0)		

Drilling Contractor:

lows/ft	etrome
N-Value (Blows/ft	Pocket Penetrome
z	ρ.

13

20

21

87/10

60

84/11

50/2

40

50/5

81

64

35

77

N/A

ter (tsf)

N/A

N/A

N/A

tsf

51 19

85 17

45

18 21

10

Unconfined Strength (Passing #200 Sieve (%	Moisture Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	Unit Dry Weight (pcf)
-----------------------	-----------------------	----------------------	--------------	---------------	------------------	-----------------------

29

NP

14 15

NP NP

Groundwater Elevation (ft) Depth (feet)	Sample Type	Graphic Log
---	-------------	-------------

10

- 20

25

30

35

40

45

50

55

60

65

Y

Material	Description

mottled	
Very Stiff, light tan, red and brown, Lean Clay (CL), mottled	
	mottled Very Stiff, light tan, red and brown, Lean Clay

Very Dense, red, brown and gray, Silty Sand (SM)	

Very Dense, light gray, Silty Sand (SM)

Hard, gray and dark gray, Lean Clay (CL), mottled, laminated
Black, Lignite

<u></u>	
Hard, dark gray, Lean Clay (CL), laminated, trace gypsum	mottled,

laminated			

- attempted, no recovery

Boring terminated at 62 feet.

95	18	43	17	26	

Additional	information/	Comments
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Logger: D. Diduch

Notes/Comments: Seepage encountered at 28 ft during drilling. Water level at 28 feet upon completion. N/A: Not Attempted



LOG OF BORING B8

Project Name:	Pirkey - 2019 Landfill Expansion
Th. 1 . 1	77 - 11

Project Location: Hallsville, Texas Drilling Contractor: C&S Lease

Project No.:

2016-011

GPS Coordinates:	N32° 27' 5.7"	
or o dooramates.	1102 2/ 5:/	

W94° 29' 39.6"

Drill Date(s):

09/13/2016

Sui	Surface Elevation: N/A					
Dri	Drilling Method: HSA					
Groundwater Elevation (ft)	Depth (feet)	Sample Type	Graphic Log			

Material Description

Loose, red, tan, and brown, Clayey Sand (SC)

- medium dense, light gray, red and tan

Pocket Penetrometer (tsf) Unconfined Strength (tsf) Passing #200 Sieve (%) Moisture Content (%) Unit Dry Welght (pcf) N-Value (Blows/ft)

> 39 13

19 12

1		-5	
		10	
	▼	- 15	

	- - - - 20	Very Stiff, gray and brown, Lean Clay (CL),	22	
¥	- - - 15	Selow ork	15	

Plasticity Index Liquid Limit

Plastic Limit

•	15	\bowtie		15								
	- - 20	\times	Very Stiff, gray and brown, Lean Clay (CL), mottled, laminated	22		91	25	42	16	26		
	25	\times	- hard below 23 ft	79								
	- - - 30			79/11					l			
	35	\times	Very Dense, dark gray, Silty Sand (SM)	76/11		43	30	NP	NP	NP		
	- 4 0	\times	Very Stiff, gray and dark gray, Lean Clay (CL), mottled, laminated	21		97	23					
	45	×	Stiff, gray and dark gray, Sandy Lean Clay (CL), mottled, with occasional lignitic seams	22	1.5	59	25	33	18	15	93	
	- 50	×	- attempted, no recovery	16	N/A	48	30					
	- 55	\times		22								
-	- 60	>	- laminated at 58 ft	29		62	25	44	21	23		
Ė	65		Boring terminated at 60 feet.					İ				

6

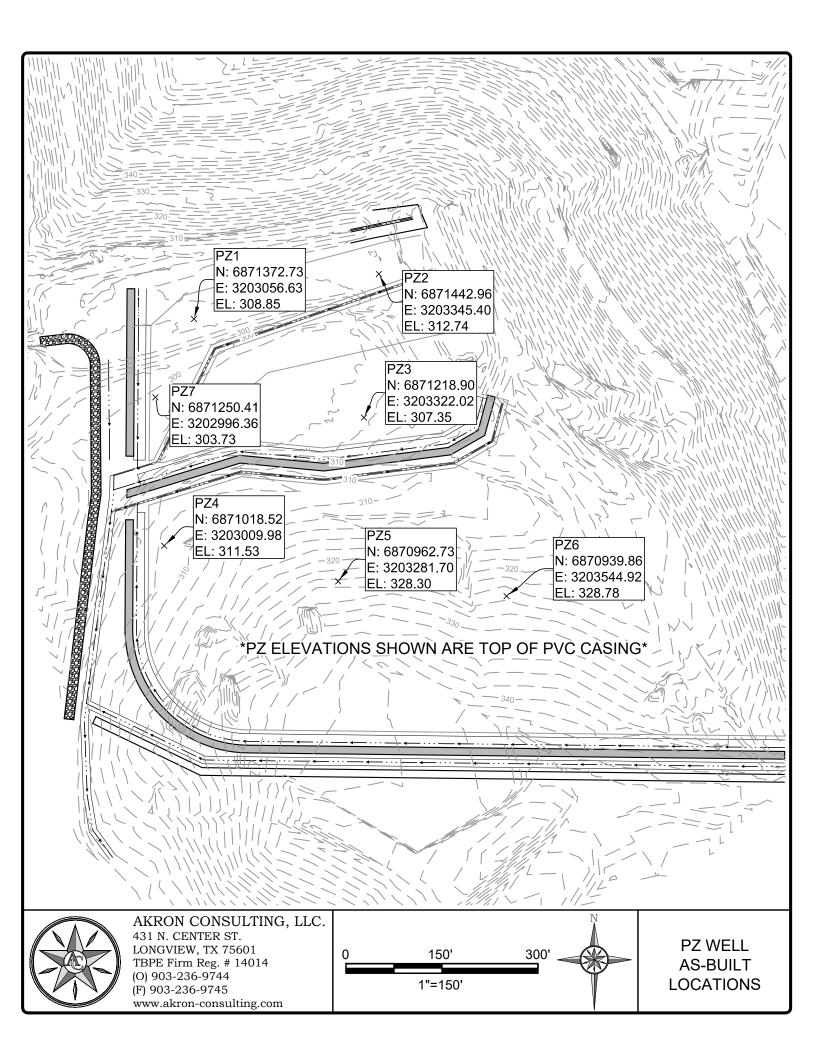
7

2.0

Additional Information/Comments:

Logger: D. Diduch

Notes/Comments: Seepage encountered at 13 ft during drilling. Water level at 13 feet upon completion.



Latitude:

Owner: American Electric Power Company Owner Well #: PZ-1

Address: 502 N. Allen Strreet Grid #: 35-37-4

Shreveport, LA 71101

Well Location: 2400 Farm Road

Hallsville, TX 75650

32° 27' 11.79" N

Longitude: 094° 29' 48.1" W

Well County: Harrison Elevation: No Data

Type of Work: New Well Proposed Use: Piezometer

Drilling Start Date: 6/14/2018 Drilling End Date: 6/14/2018

 Diameter (in.)
 Top Depth (ft.)
 Bottom Depth (ft.)

 Borehole:
 8.25
 0
 14

Drilling Method: Hollow Stem Auger

Borehole Completion: Filter Packed

Filter Pack Intervals:

Top Depth (ft.)

Bottom Depth (ft.)

Filter Material

Size

Sand

20/40

Annular Seal Data:

Top Depth (ft.)

Bottom Depth (ft.)

Description (number of sacks & material)

Cement 1 Bags/Sacks

1 3 Bentonite 2 Bags/Sacks

Seal Method: Poured Distance to Property Line (ft.): No Data

Sealed By: **Driller**Distance to Septic Field or other

concentrated contamination (ft.): No Data

Distance to Septic Tank (ft.): No Data

. , ,

Method of Verification: No Data

Surface Completion: Alternative Procedure Used Surface Completion by Driller

Water Level: No Data

Packers: No Data

Type of Pump: No Data

Water Quality:

No Data

No Data

Water Type

Chemical Analysis Made: No

Did the driller knowingly penetrate any strata which contained injurious constituents?: **No**

Certification Data: The driller certified that the driller drilled this well (or the well was drilled under the

driller's direct supervision) and that each and all of the statements herein are true and correct. The driller understood that failure to complete the required items will result in

the report(s) being returned for completion and resubmittal.

Company Information: C&S Lease

1873 FM 1252 E Kilgore, TX 75663

Driller Name: Buford E. Collier License Number: 50089

Apprentice Name: Michael Aaron Dodson Apprentice Number: 59693

Comments: No Data

Lithology: DESCRIPTION & COLOR OF FORMATION MATERIAL

Casing: BLANK PIPE & WELL SCREEN DATA

Top (ft.)	Bottom (ft.)	Description
0	5	Red Soft Clay
5	10	Very Soft Red/Grey Clay
10	14	Very Soft Brown Sandy Clay

Dla (in.)	Туре	Material	Sch./Gage	Top (ft.)	Bottom (ft.)
2	Riser	New Plastic (PVC)	40	0	4
2	Screen	New Plastic (PVC)	40 0.010	4	14

IMPORTANT NOTICE FOR PERSONS HAVING WELLS DRILLED CONCERNING CONFIDENTIALITY

TEX. OCC. CODE Title 12, Chapter 1901.251, authorizes the owner (owner or the person for whom the well was drilled) to keep information in Well Reports confidential. The Department shall hold the contents of the well log confidential and not a matter of public record if it receives, by certified mail, a written request to do so from the owner.

Please include the report's Tracking Number on your written request.

Latitude:

Elevation:

32° 27' 12.36" N

No Data

Owner: American Electric Power Company Owner Well #: PZ-2

Address: 502 N. Allen Strreet Grid #: 35-37-4

Shreveport, LA 71101

Harrison

Well County:

Well Location: 2400 Farm Road Hallsville, TX 75650 Longitude: 094° 29' 44.64" W

3

Type of Work: New Well Proposed Use: Piezometer

Drilling Start Date: 6/14/2018 Drilling End Date: 6/14/2018

 Diameter (in.)
 Top Depth (ft.)
 Bottom Depth (ft.)

 Borehole:
 8.25
 0
 14

Drilling Method: Hollow Stem Auger

Borehole Completion: Filter Packed

Filter Pack Intervals:

Top Depth (ft.)

Bottom Depth (ft.)

Filter Material

Size

Sand

20/40

Annular Seal Data:

Top Depth (ft.)

Bottom Depth (ft.)

Description (number of sacks & material)

Cement 1 Bags/Sacks

1 3 Bentonite 2 Bags/Sacks

Seal Method: Poured Distance to Property Line (ft.): No Data

Sealed By: **Driller** Distance to Septic Field or other

concentrated contamination (ft.): No Data

Distance to Septic Tank (ft.): No Data

Method of Verification: No Data

Surface Completion: Alternative Procedure Used Surface Completion by Driller

Water Level: No Data

Packers: No Data

Type of Pump: No Data

Chemical Analysis Made: No

Did the driller knowingly penetrate any strata which contained injurious constituents?: **No**

Certification Data: The driller certified that the driller drilled this well (or the well was drilled under the

driller's direct supervision) and that each and all of the statements herein are true and correct. The driller understood that failure to complete the required items will result in

the report(s) being returned for completion and resubmittal.

Company Information: C&S Lease

1873 FM 1252 E Kilgore, TX 75663

Driller Name: Buford E. Collier License Number: 50089

Apprentice Name: Michael Aaron Dodson Apprentice Number: 59693

Comments: No Data

Lithology: DESCRIPTION & COLOR OF FORMATION MATERIAL

Casing: BLANK PIPE & WELL SCREEN DATA

Top (ft.)	Bottom (ft.)	Description
0	5	Red Soft Clay
5	10	Very Soft Red/Grey Clay
10	14	Very Soft Brown Sandy Clay

Dla (in.)	Туре	Material	Material Sch./Gage		Bottom (ft.)
2	Riser	New Plastic (PVC)	40	0	4
2	Screen	New Plastic (PVC)	40 0.010	4	14

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Please include the report's Tracking Number on your written request.

Owner: American Electric Power Company Owner Well #: PZ-3

Address: 502 N. Allen Strreet Grid #: 35-37-4

Shreveport, LA 71101

Well Location: 2400 Farm Road Latitude: 32° 27' 10.18" N

Hallsville, TX 75650 Longitude: 094° 29' 45.15" W

Well County: Harrison Elevation: No Data

Type of Work: New Well Proposed Use: Piezometer

Drilling Start Date: 6/14/2018 Drilling End Date: 6/14/2018

 Diameter (in.)
 Top Depth (ft.)
 Bottom Depth (ft.)

 Borehole:
 8.25
 0
 14

Drilling Method: Hollow Stem Auger

Borehole Completion: Filter Packed

Filter Pack Intervals:

Top Depth (ft.)

Bottom Depth (ft.)

Filter Material

Size

Sand

20/40

Annular Seal Data:

Top Depth (ft.)

Bottom Depth (ft.)

Description (number of sacks & material)

Cement 1 Bags/Sacks

1 3 Bentonite 2 Bags/Sacks

Seal Method: Poured Distance to Property Line (ft.): No Data

Sealed By: **Driller** Distance to Septic Field or other

concentrated contamination (ft.): No Data

Distance to Septic Tank (ft.): No Data

Method of Verification: No Data

Surface Completion: Alternative Procedure Used Surface Completion by Driller

Water Level: No Data

Packers: No Data

Type of Pump: No Data

Chemical Analysis Made: No

Did the driller knowingly penetrate any strata which contained injurious constituents?: **No**

Certification Data: The driller certified that the driller drilled this well (or the well was drilled under the

driller's direct supervision) and that each and all of the statements herein are true and correct. The driller understood that failure to complete the required items will result in

the report(s) being returned for completion and resubmittal.

Company Information: C&S Lease

1873 FM 1252 E Kilgore, TX 75663

Driller Name: Buford E. Collier License Number: 50089

Apprentice Name: Michael Aaron Dodson Apprentice Number: 59693

Comments: No Data

Lithology: DESCRIPTION & COLOR OF FORMATION MATERIAL

Casing: BLANK PIPE & WELL SCREEN DATA

Top (ft.)	Bottom (ft.)	Description
0	5	Red Soft Clay
5	10	Very Soft Red/Grey Clay
10	14	Very Soft Brown Sandy Clay

Dla (in.)	Туре	Material	Sch./Gage	Top (ft.)	Bottom (ft.)
2	Riser	New Plastic (PVC)	40	0	4
2	Screen	New Plastic (PVC)	40 0.010	4	14

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Please include the report's Tracking Number on your written request.

Owner: American Electric Power Company Owner Well #: PZ-4

Address: 502 N. Allen Strreet Grid #: 35-37-4

Shreveport, LA 71101

Well Location: 2400 Farm Road

Hallsville, TX 75650

Latitude: 32° 27' 08.3" N

Longitude: 094° 29' 48.73" W

Well County: Harrison Elevation: No Data

Type of Work: New Well Proposed Use: Piezometer

Drilling Start Date: 6/15/2018 Drilling End Date: 6/15/2018

 Diameter (in.)
 Top Depth (ft.)
 Bottom Depth (ft.)

 Borehole:
 8.25
 0
 14

Drilling Method: Hollow Stem Auger

Borehole Completion: Filter Packed

Filter Pack Intervals:

Top Depth (ft.)

Bottom Depth (ft.)

Filter Material

Size

Sand

20/40

Annular Seal Data:

Top Depth (ft.)

Bottom Depth (ft.)

Description (number of sacks & material)

Cement 1 Bags/Sacks

1 3 Bentonite 2 Bags/Sacks

Seal Method: Poured Distance to Property Line (ft.): No Data

Sealed By: **Driller** Distance to Septic Field or other

concentrated contamination (ft.): No Data

Distance to Septic Tank (ft.): No Data

Method of Verification: No Data

Surface Completion: Alternative Procedure Used Surface Completion by Driller

Water Level: No Data

Packers: No Data

Type of Pump: No Data

Chemical Analysis Made: No

Did the driller knowingly penetrate any strata which contained injurious constituents?: **No**

Certification Data: The driller certified that the driller drilled this well (or the well was drilled under the

driller's direct supervision) and that each and all of the statements herein are true and correct. The driller understood that failure to complete the required items will result in

the report(s) being returned for completion and resubmittal.

Company Information: C&S Lease

1873 FM 1252 E Kilgore, TX 75663

Driller Name: Buford E. Collier License Number: 50089

Apprentice Name: Michael Aaron Dodson Apprentice Number: 59693

Comments: No Data

Lithology: DESCRIPTION & COLOR OF FORMATION MATERIAL

Casing: BLANK PIPE & WELL SCREEN DATA

Top (ft.)	Bottom (ft.)	Description	
0	5	Red Soft Clay	
5	10	Very Soft Red/Grey Clay	
10 14		Very Soft Brown Sandy Clay	

Dla (in.)	Туре	Material	Sch./Gage	Top (ft.)	Bottom (ft.)
2	Riser	New Plastic (PVC)	40	0	4
2	Screen	New Plastic (PVC)	40 0.010	4	14

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Please include the report's Tracking Number on your written request.

Owner: Owner Well #: PZ-5 **American Electric Power Company**

Address: 502 N. Allen Strreet Grid #: 35-37-4

Shreveport, LA 71101

32° 27' 07.7" N Well Location: 2400 Farm Road

Hallsville, TX 75650 Longitude: 094° 29' 45.72" W

Latitude:

Well County: Harrison Elevation: No Data

Type of Work: **New Well** Proposed Use: **Piezometer**

Drilling Start Date: 6/15/2018 Drilling End Date: 6/15/2018

Diameter (in.) Top Depth (ft.) Bottom Depth (ft.) Borehole: 8.25 20 0

Hollow Stem Auger Drilling Method:

Borehole Completion: **Filter Packed**

Filter Material Size Top Depth (ft.) Bottom Depth (ft.) Filter Pack Intervals: 8 20 Sand 20/40

Top Depth (ft.) Bottom Depth (ft.) Description (number of sacks & material) Annular Seal Data: 0 1 Cement 1 Bags/Sacks 8 1 **Bentonite 4 Bags/Sacks**

Seal Method: Poured Distance to Property Line (ft.): No Data

Sealed By: **Driller** Distance to Septic Field or other

concentrated contamination (ft.): No Data

Distance to Septic Tank (ft.): No Data

Method of Verification: No Data

Surface Completion: **Alternative Procedure Used** Surface Completion by Driller

Water Level: No Data

Packers: No Data

Type of Pump: No Data

Chemical Analysis Made: No

Did the driller knowingly penetrate any strata which contained injurious constituents?: **No**

Certification Data: The driller certified that the driller drilled this well (or the well was drilled under the

driller's direct supervision) and that each and all of the statements herein are true and correct. The driller understood that failure to complete the required items will result in

the report(s) being returned for completion and resubmittal.

Company Information: C&S Lease

1873 FM 1252 E Kilgore, TX 75663

Driller Name: Buford E. Collier License Number: 50089

Apprentice Name: Michael Aaron Dodson Apprentice Number: 59693

Comments: No Data

Lithology: DESCRIPTION & COLOR OF FORMATION MATERIAL

Casing: BLANK PIPE & WELL SCREEN DATA

Top (ft.)	Bottom (ft.)	Description
0	5	Red Soft Sandy Clay
5	10	Very Soft Red/Brown Clay
10	15	Very Soft Red/Tan Sandy Clay
15	20	Tan/Red Silty Sand

Dla (in.)	Туре	Material	Sch./Gage	Top (ft.)	Bottom (ft.)
2	Riser	New Plastic (PVC)	40	0	10
2	Screen	New Plastic (PVC)	40 0.010	10	20

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Please include the report's Tracking Number on your written request.

Owner: American Electric Power Company Owner Well #: PZ-6

Address: 502 N. Allen Strreet Grid #: 35-37-4

Shreveport, LA 71101

Well Location:

2400 Farm Road Hallsville, TX 75650

Hallsville, TX 75650 Longitude: 094° 29' 42.56" W

Latitude:

32° 27' 07.69" N

Well County: Harrison Elevation: No Data

Type of Work: New Well Proposed Use: Piezometer

Drilling Start Date: 6/15/2018 Drilling End Date: 6/15/2018

 Diameter (in.)
 Top Depth (ft.)
 Bottom Depth (ft.)

 Borehole:
 8.25
 0
 20

Drilling Method: Hollow Stem Auger

Borehole Completion: Filter Packed

Filter Pack Intervals:

8 20 Sand 20/40

Annular Seal Data:

Top Depth (ft.)

Bottom Depth (ft.)

Description (number of sacks & material)

Cement 1 Bags/Sacks

1 8 Bentonite 4 Bags/Sacks

Seal Method: Poured Distance to Property Line (ft.): No Data

Sealed By: **Driller** Distance to Septic Field or other

concentrated contamination (ft.): No Data

Distance to Septic Tank (ft.): No Data

Method of Verification: No Data

Surface Completion: Alternative Procedure Used Surface Completion by Driller

Water Level: No Data

Packers: No Data

Type of Pump: No Data

Water Quality:

No Data

No Data

Water Type

Chemical Analysis Made: No

Did the driller knowingly penetrate any strata which contained injurious constituents?: **No**

Certification Data: The driller certified that the driller drilled this well (or the well was drilled under the

driller's direct supervision) and that each and all of the statements herein are true and correct. The driller understood that failure to complete the required items will result in

the report(s) being returned for completion and resubmittal.

Company Information: C&S Lease

1873 FM 1252 E Kilgore, TX 75663

Driller Name: Buford E. Collier License Number: 50089

Apprentice Name: Michael Aaron Dodson Apprentice Number: 59693

Comments: No Data

Lithology: DESCRIPTION & COLOR OF FORMATION MATERIAL

Casing: BLANK PIPE & WELL SCREEN DATA

Top (ft.)	Bottom (ft.)	Description
0	5	Red Soft Sandy Clay
5	10	Very Soft Red/Brown Clay
10	15	Very Soft Red/Tan Sandy Clay
15	20	Tan/Red Silty Sand

Dla (in.)	Туре	Material	Sch./Gage	Top (ft.)	Bottom (ft.)
2	Riser	New Plastic (PVC)	40	0	10
2	Screen	New Plastic		10	20

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Please include the report's Tracking Number on your written request.

Owner: American Electric Power Company Owner Well #: PZ-7

Address: 502 N. Allen Strreet Grid #: 35-37-4

Shreveport, LA 71101

Well Location: 2400 Farm Road Latitude: 32° 27' 10.81" N

Hallsville, TX 75650 Longitude: 094° 29' 48.7" W

Well County: Harrison Elevation: No Data

Type of Work: New Well Proposed Use: Piezometer

Drilling Start Date: 6/14/2018 Drilling End Date: 6/14/2018

 Diameter (in.)
 Top Depth (ft.)
 Bottom Depth (ft.)

 Borehole:
 8.25
 0
 14

Drilling Method: Hollow Stem Auger

Borehole Completion: Filter Packed

Filter Pack Intervals:

Top Depth (ft.)

Bottom Depth (ft.)

Filter Material

Size

Sand

20/40

Annular Seal Data:

Top Depth (ft.)

Bottom Depth (ft.)

Description (number of sacks & material)

Cement 1 Bags/Sacks

1 3 Bentonite 2 Bags/Sacks

Seal Method: Poured Distance to Property Line (ft.): No Data

Sealed By: **Driller** Distance to Septic Field or other

concentrated contamination (ft.): No Data

Distance to Septic Tank (ft.): No Data

Method of Verification: No Data

Surface Completion: Alternative Procedure Used Surface Completion by Driller

Water Level: No Data

Packers: No Data

Type of Pump: No Data

Chemical Analysis Made: No

Did the driller knowingly penetrate any strata which contained injurious constituents?: **No**

Certification Data: The driller certified that the driller drilled this well (or the well was drilled under the

driller's direct supervision) and that each and all of the statements herein are true and correct. The driller understood that failure to complete the required items will result in

the report(s) being returned for completion and resubmittal.

Company Information: C&S Lease

1873 FM 1252 E Kilgore, TX 75663

Driller Name: Buford E. Collier License Number: 50089

Apprentice Name: Michael Aaron Dodson Apprentice Number: 59693

Comments: No Data

Lithology: DESCRIPTION & COLOR OF FORMATION MATERIAL

Casing: BLANK PIPE & WELL SCREEN DATA

Top (ft.)	Bottom (ft.)	Description	
0	5	Red Soft Clay	
5	10	Very Soft Red/Grey Clay	
10	14	Very Soft Brown Sandy Clay	

Dla (in.)	Туре	Material	Sch./Gage	Top (ft.)	Bottom (ft.)
2	Riser	New Plastic (PVC)	40	0	4
2	Screen	New Plastic (PVC)	40 0.010	4	14

IMPORTANT NOTICE FOR PERSONS HAVING WELLS DRILLED CONCERNING CONFIDENTIALITY

TEX. OCC. CODE Title 12, Chapter 1901.251, authorizes the owner (owner or the person for whom the well was drilled) to keep information in Well Reports confidential. The Department shall hold the contents of the well log confidential and not a matter of public record if it receives, by certified mail, a written request to do so from the owner.

Please include the report's Tracking Number on your written request.