



2020 DAM AND DIKE INSPECTION REPORT GEVR-20-008

BOTTOM ASH POND NORTHEASTERN 3&4 PLANT PUBLIC SERVICE COMPANY OF OKLAHOMA

OOLOGAH, OKLAHOMA

Prepared for:



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2020 Annual Dam and Dike Inspection Report

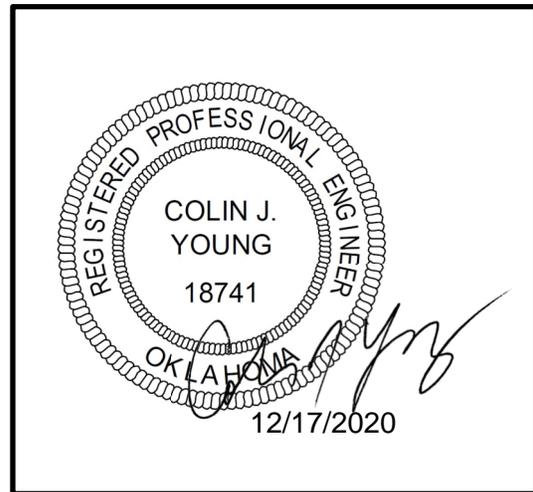
Northeastern 3&4 Plant

Bottom Ash Pond

Document Number: GEVR-20-008

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I certify to the best of my knowledge, information and belief the information contained in this report meets the requirements of OAC § 252:517-13-4.

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1.0 INTRODUCTION

This report was prepared by Freese and Nichols, Inc. to fulfill requirements of OAC 252:517-13-4 and to provide the Northeastern 3&4 Plant an evaluation of the facility.

Mr. Colin Young, P.E. and Tyler Ogle performed the 2020 inspection of the Bottom Ash Pond at the Northeastern 3&4 Power Station. Greg Carter, P.E., AEP Regional Engineering, participated in the inspection and provided access. This report is a summary of the inspection and an assessment of the general condition of the facility.

The inspection was performed on November 06, 2020. Weather conditions were sunny and the temperature was in the mid 60's (°F). According to observed precipitation maps by the National Weather Service, a cumulative total of approximately 0.5 inches of rainfall was measured over the 7 days prior to the inspection.

2.0 DESCRIPTION OF IMPOUNDMENT

Figure 1 provides a plan view, in the form of a satellite image, of the Bottom Ash Pond, the embankment structure or dam, pertinent dam features, and the dam's appurtenances. The dam is a 4,200-foot long cross-valley fill on an unnamed tributary to Fourmile Creek. The dam is roughly U-shaped and has been divided into north, west, and south embankments for reference in this inspection report.

There is no principal spillway at the bottom ash pond; water is typically recirculated back to the power plant for reuse. The auxiliary spillway at the bottom ash pond is a concrete overflow structure with a design crest elevation of 625.0 feet. Overflow from the spillway discharges to a low area and then flows through culverts under a railroad and off site. Figure 1 shows the spillway and discharge culvert locations.

The dam was designed with a toe drain along the west and south embankments. This drainage system consists of a 1.5-foot-thick sand and gravel drainage blanket layer that extends along the subgrade from the toe toward the centerline of the embankment as shown on the design drawings. The drainage blanket is connected to a gravel and sand bedding layer, 9-inches in thickness, at the toe that runs 12 feet up the slope from the toe and is overlain by a 1-foot layer of riprap. The toe drain was designed to drain seepage from the dam at any point along its length; i.e., there are no seepage collection pipes to discharge seepage at specific locations. Therefore, seepage will tend to collect and discharge at the lowest elevation along the toe. This area is near the western end of the south embankment at the location of the pre-existing natural streambed, where a pipe was installed under the access road.

A railroad track used for coal deliveries to the plant runs along the crest of the west and south dikes.

3.0 REVIEW OF AVAILABLE INFORMATION (252:517-13-4(b)(1)(A))

A review of available information regarding the status and condition of the Bottom Ash Pond has been conducted. This includes files available in the operating record, such as design and construction information, previous periodic structural stability assessments, previous 7-day inspection reports, 30-day data collection reports, and previous annual inspections. Based on the review of the data there were no signs of actual or potential structural weaknesses or adverse conditions.

4.0 INSPECTION (252:517-13-4(b)(1)(B))

4.1 CHANGES IN GEOMETRY SINCE LAST INSPECTION (252:517-13-4(b)(2)(A))

No modifications have been made to the geometry of the Bottom Ash Pond since the previous annual inspection. The geometry of the impoundment has remained essentially unchanged.

4.2 INSTRUMENTATION (252:517-13-4(b)(2)(A))

There are two piezometers and a seepage collection pipe as part of the instrumentation for this facility. The locations of the instrumentations are shown on Figure 2. The maximum and minimum recorded readings of each piezometer since the previous annual inspection is shown in Table 1 below. The readings collected since the last inspection were all within their normal safe operating ranges. MW-01 is located on the crest of the dam and MW-02 is located at the toe of the dam. The water level in the pond ranged from elevation 624.5 – 622.3.

Additionally, the seepage collected at the toe of the south embankment is measured at the culvert. Since the installation of the new culvert in 2016, there has been no measurable flow from the culvert.

Table 1

INSTRUMENTATION DATA			
Bottom Ash Pond			
Instrument	Type	Max/Min Reading since last annual inspection	Date of readings
MW-01	Piezometer	609.52/608.6	1-8-2020 /10-7-2020
MW-02	Piezometer	599.67/598.82	1-29-2020/9-16-2020

4.3 IMPOUNDMENT CHARACTERISTICS (252:517-13-4(b)(2)(C))

Table 2 is a summary of the minimum, maximum, and present depth and elevation of the impounded water and Coal Combustion Residuals since the previous annual inspection; the storage capacity of the impounding structure at the time of the inspection; and the approximate volume of the impounded water and CCR at the time of the inspection. (Most of the bottom ash settles out is a very small area that is periodically excavated and either beneficially used or placed in the on-site landfill. Therefore, the depth range of ash does not vary.

Table 2

IMPOUNDMENT CHARACTERISTICS	
Bottom Ash Pond (crest elev: 630.0; lowest 604.0)	
Approximate Minimum depth of impounded water since last annual inspection	18.5 feet. (622.5)
Approximate Maximum depth of impounded water since last annual inspection	21.0 feet. (625.0)
Approximate Present depth of impounded water at the time of the inspection	18.5 feet. (622.5)
Approximate Minimum depth of CCR since last annual inspection	5 feet.

Approximate Maximum depth of CCR since last annual inspection	5 feet.
Approximate Present depth of CCR at the time of the inspection	5 feet.
Storage Capacity of impounding structure at the time of the inspection	183 acre-feet.
Approximate volume of impounded water at the time of the inspection	183 acre-feet.
Approximate volume of CCR at the time of the inspection	145 acre-feet (el 625 -630)

4.4 DEFINITIONS OF VISUAL OBSERVATIONS AND DEFICIENCIES

This summary of the visual observations uses terms to describe the general appearance or condition of an observed item, activity or structure. The meaning of these terms is as follows:

- Good:** A condition or activity that is generally better or slightly better than what is minimally expected or anticipated from a design or maintenance point of view.
- Fair/Satisfactory:** A condition or activity that generally meets what is minimally expected or anticipated from a design or maintenance point of view.
- Poor:** A condition or activity that is generally below what is minimally expected or anticipated from a design or maintenance point of view.
- Minor:** A reference to an observed item (e.g., erosion, seepage, vegetation, etc.) where the current condition is below what is normal or desired, but which is not currently causing concern from a structure safety or stability point of view.
- Significant:** A reference to an observed item (e.g. erosion, seepage, vegetation, etc.) where the current maintenance has neglected to improve the condition. Usually conditions that have been identified in the previous inspections, but have not been corrected.
- Excessive:** A reference to an observed item (e.g., erosion, seepage, vegetation, etc.) where the current condition is above or worse than what is normal or desired, and which may have affected the ability of the observer to properly evaluate the structure or particular area being observed or which may be a concern from a structure safety or stability point of view.

This document also uses the definition of a “deficiency” as referenced in the CCR rule section §257.83(b)(5) Inspection Requirements for CCR Surface Impoundments. This definition has been assembled using the CCR rule preamble as well as guidance from MSHA, “Qualifications for Impoundment Inspection” CI-31, 2004. These guidance documents further elaborate on the definition of deficiency. Items not defined by deficiency are considered maintenance or items to be monitored.

A “deficiency” is some evidence that a problem has developed that could impact the structural integrity of the structure. There are four general categories of deficiencies. These four categories are described below:

1. Uncontrolled Seepage

Uncontrolled seepage is seepage that is not behaving as the design engineer has intended. An example of uncontrolled seepage is seepage that comes through or around the embankment and is not picked up and safely carried off by a drain. Seepage that is collected by a drain can still be uncontrolled if it is not safely collected and transported. Seepage that is not clear and is turbid would also be considered as uncontrolled. Seepage that is unable to be measured and/or observe it is considered uncontrolled seepage.

Note: Wet or soft areas are not considered as uncontrolled seepage, but can lead to this type of deficiency. These areas should be monitored more frequently.

2. Displacement of the Embankment

Displacement of the embankment is large scale movement of part of the dam. Common signs of displacement are cracks, scraps, bulges, depressions, sinkholes and slides.

3. Blockage of Control Features

Blockage of Control Features is the restriction of flow at spillways, decant or pipe spillways, or drains.

4. Erosion

Erosion is the gradual movement of surface material by water, wind or ice. Erosion is considered a deficiency when it is more than a minor routine maintenance item.

4.5 VISUAL INSPECTION (252:517-13-4(b)(1)(B))

A visual inspection of the Bottom Ash Pond was conducted to identify any signs of distress or malfunction of the impoundment and appurtenant structures. Specific items inspected included all structural elements of the dam such as upstream and downstream slopes, crest, and toe; as well as appurtenances.

Selected inspection photos are included in Attachment A. Additional pictures taken during the inspection can be made available upon request.

North Embankment

1. The intake structure was in good structural condition. The metal platform and concrete structure show no signs of deterioration. The intake screen was clear of debris. The staff gauge was in good condition.
2. The upstream slope shows no signs of sloughing or bulges. The riprap protection along the slope is in good condition and has not deteriorated. There was no grassy vegetation growing within the riprap. (photo 2 and 3)
3. The downstream slope of the embankment was well vegetated. The grass along the downstream slope was recently mowed. There were no signs of sloughing or other slope movement. (photo 1)
4. The crest appeared in good and stable condition with no significant settlement, misalignment, or noticeable sign of distress.

Auxiliary Spillway

1. The concrete control section and discharge chute were in good condition and clear of any heavy vegetation or debris that would restrict flow. (photo 4 and 5)
2. There were no signs of movement or misalignment along any of the construction joints except for the joint at the top of the chute. This occurred some time ago and the condition has remained stable based on past inspections. There was no seepage along the contacts between the concrete training walls and downstream slope of the embankment, nor at the end of the concrete chute.
3. The energy dissipater baffles at the toe of the spillway were clear of sediment and were in good condition. (photo 5)
4. The caulking at the concrete joints was in good condition.

West Embankment

1. The upstream slope of this embankment was in good condition. The riprap was free of any vegetation. (photo 6)
2. The area beyond the toe of the embankment has been well maintained to prevent heavy and/or woody vegetation within the regulatory 30-foot setback. The downstream slope is well vegetated and maintained (photo 7). There was no seepage noted along the toe of the West embankment.
3. The crest and railroad tracks do not show any signs of settlement or misalignment. The tracks are inspected monthly by an independent company to ensure no misalignment or settlement.
4. Two rodent holes were observed just above the riprap on the downstream slope. (photo 8) (see Location Map).

South Embankment

5. The upstream slope of the South Dike embankment was in good condition. There were no signs of bulges, cracks, sloughing or other deficiencies. There is vegetation growing in the riprap on the southeast upstream embankment. The crest and railroad tracks do not show any signs of settlement or misalignment. As noted above, the tracks are inspected monthly by an outside company to ensure no misalignment or settlement. (photo 9)
6. The downstream slope is well vegetated and maintained. There was no observed seepage, wet or damp areas along the slope. (photos 10 and 11)
7. One rodent hole was observed on the downstream slope just above the riprap.
8. The seepage collection blanket appears to be functioning as designed. Seepage is directed to the low area below the embankment and is typically discharged through a pipe below an access drive. No seepage or moisture was observed at the outlet of the pipe during the inspection (photo 12).
9. The area beyond the toe of the embankment has been well maintained to prevent heavy and/or woody vegetation within the regulatory 30-foot setback.

4.6 CHANGES THAT AFFECT STABILITY OR OPERATION (252:517-13-4(b)(2)(G))

Based on field observations there were no changes to the Bottom Ash Pond since the last annual inspection that would affect the stability or operation of the impounding structure.

5.0 SUMMARY OF FINDINGS

5.1 GENERAL OBSERVATIONS

The following general observations were identified during the visual inspection:

- 1) Overall, the facility is in good condition. The impoundment is functioning as intended with no signs of potential structural weakness or conditions, which are disrupting to the safe operation of the impoundment.
- 2) The slopes and crest of the embankment were generally in good condition. The embankments did not exhibit any signs of structural weakness or instability.

5.2 MAINTENANCE ITEMS

The following maintenance items were identified during the visual inspection, see inspection map for locations:

- 1) Repair the three observed rodent holes. Mitigate holes by filling with compatible material and compaction.
- 2) Continue to clear vegetation growing in the riprap.

5.3 ITEMS TO MONITOR

The following items were identified during the visual inspection as items to be monitored.

- 1) None

5.4 DEFICIENCIES (252:517-13-4(b)(5))

There were no signs of structural weakness or disruptive conditions that were observed at the time of the inspection that would require additional investigation or remedial action. There were no deficiencies noted during this inspection or during any of the periodic 7-day inspections or 30-day data collection since the last annual inspection. A deficiency is defined as either 1) uncontrolled seepage, 2) displacement of the embankment, 3) blockage of control features, or 4) erosion, more than minor maintenance. If any of these conditions occur before the next annual inspection contact AEP Geotechnical Engineering immediately

If you have any questions regarding this report, please contact Colin Young, P.E. at (539) 302-2634.

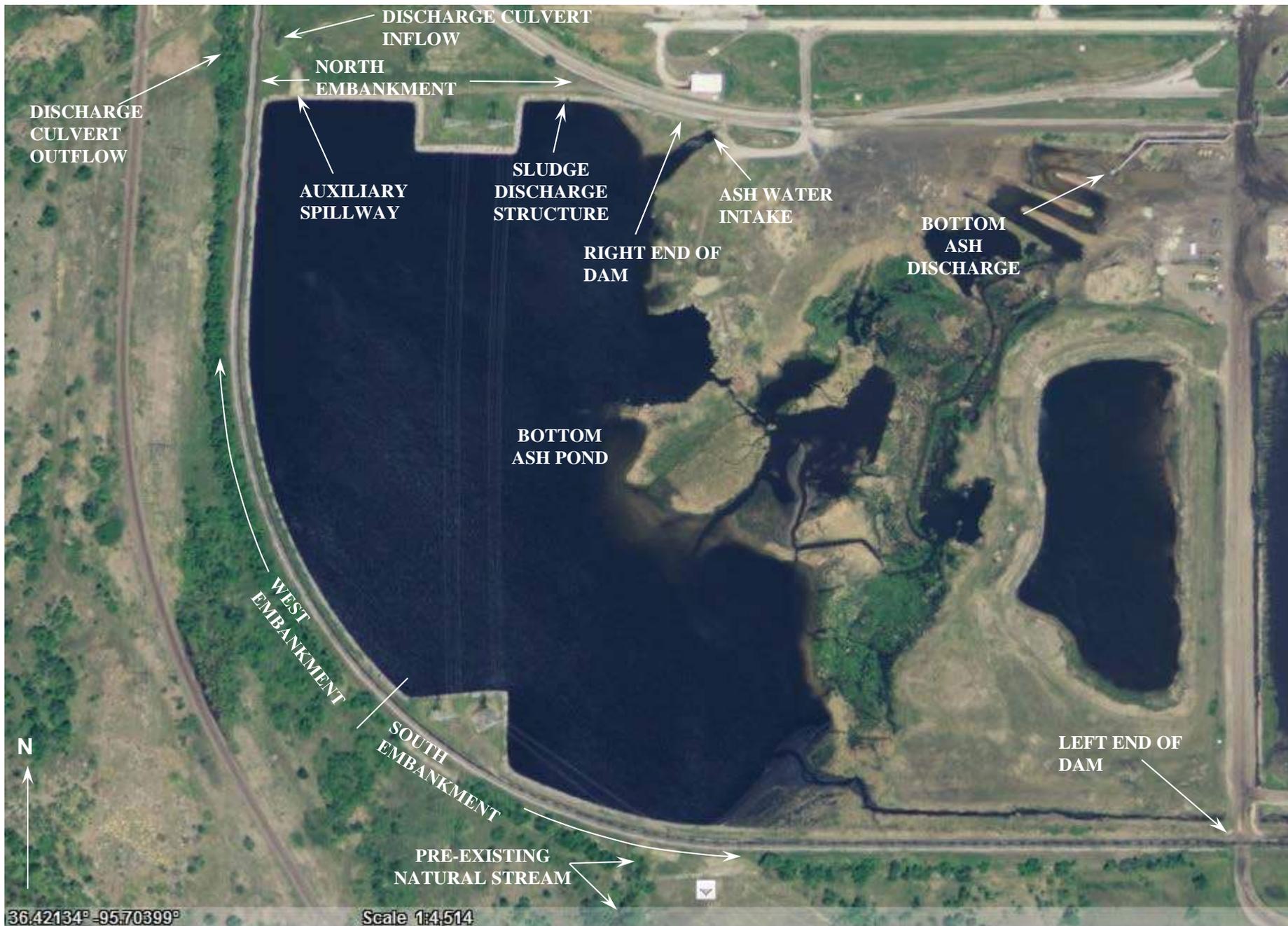
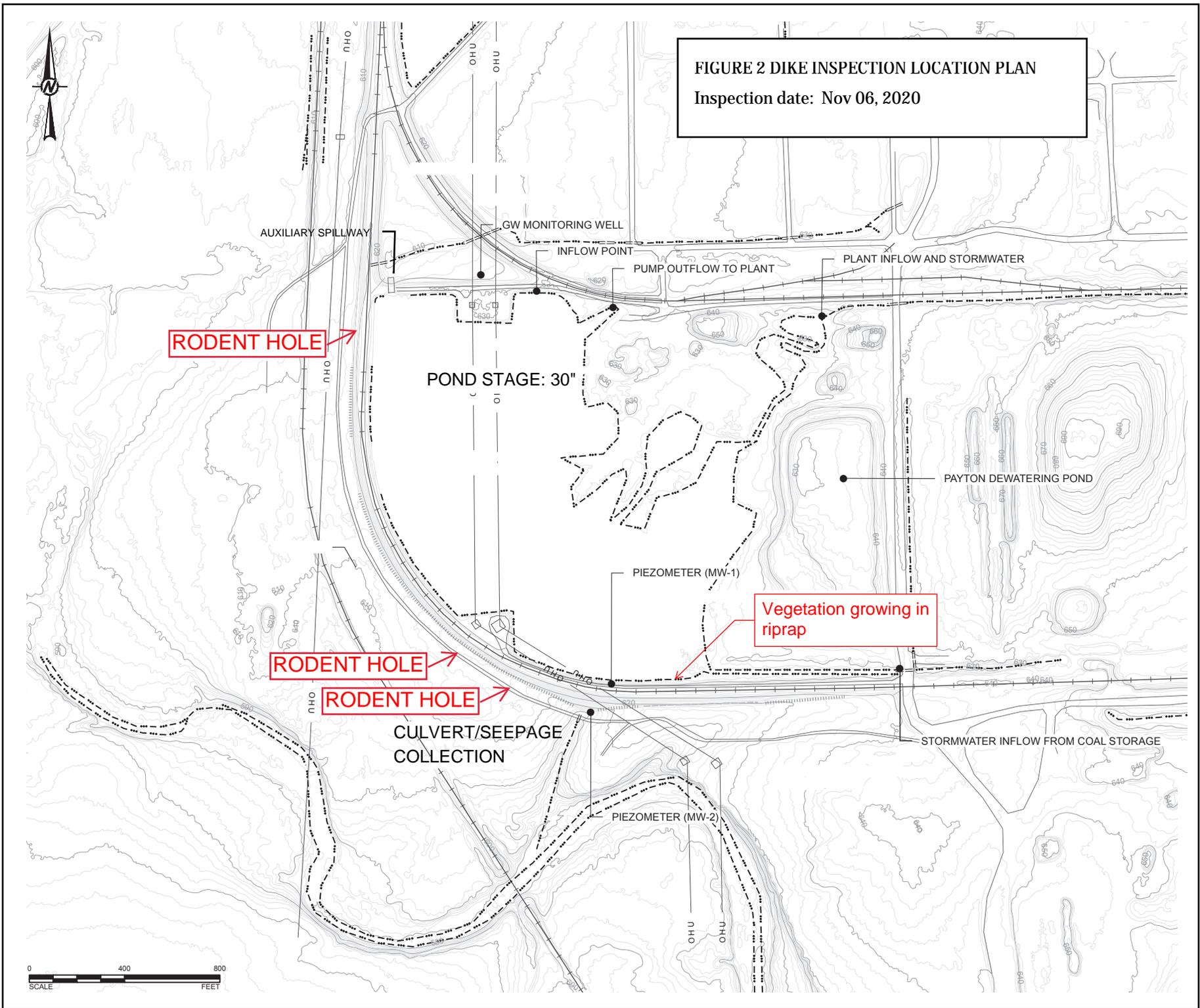


Figure 1. Satellite Image Of Bottom Ash Pond Dam, Features, And Appurtenances At Northeastern 3&4 Power Station.

FIGURE 2 DIKE INSPECTION LOCATION PLAN
Inspection date: Nov 06, 2020



ATTACHMENT A

Photos



Photo #1 - Exterior slope north dike, looking west



Photo #2 - Upstream slope of north dike, looking west



Photo #3 - Upstream slope of north dike, looking west



Photo #4 - Auxiliary spillway



Photo #5 - Spillway chute looking north



Photo #6 - Upstream slope of west embankment, looking south



Photo #7 - Downstream slope of west dike, looking northeast



Photo #8 - Rodent hole, less than 1 foot deep



Photo #9 - Vegetation growing in the riprap on the south embankment



Photo #10 - South downstream embankment looking west



Photo #11 - South downstream embankment looking west



Photo #12 - Drainage culvert at the bend between south and west embankment